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MEMORANDUM

Date: February 1, 2017 Project #: 19252

To: Jabra Khasho, PE & Ken Rencher, City of Beaverton
Jinde Zhu, Washington County Department of Land Use and Transportation

cc: Dan Grimberg, West Hills Land Development
Mike Peebles, PE and Li Alligood, OTAK

From: Julia Kuhn, PE, Chris Brehmer, PE and Zachary Bugg

Project: Lolich/Bellairs Property Development

Subject: Traffic Impact Analysis/Access Report

West Hills Land Development (herein referred to as "West Hills") is proposing to develop up to 110 single family homes and up to 200 multifamily units on the Lolich/Bellairs property, located northeast of the SW Scholls Ferry Road/SW Strobel Road/SW Vandermost Road intersection. One existing home on the property may remain in-place after neighborhood development. Construction, completion, and occupancy of the new single family homes is scheduled for 2018 but development of the 200 multifamily units will occur at a later date. A Planned Unit Development (PUD) application is being submitted for both the single family homes and multifamily dwellings at this time. As such, this traffic study addresses the cumulative transportation-related impacts of both the single family and multifamily homes.

Based on the analysis herein, the following recommendations are associated with the proposed PUD:

- Per prior traffic impact studies approved for River Terrace and South Cooper Mountain, Washington County should continue to monitor the SW Scholls Ferry Road/SW 175th Avenue/SW Roy Rogers Road intersection to determine if, and when, additional modifications are needed beyond those currently planned for construction.
- West Hills should install an eastbound left-turn lane, westbound left-turn lane, a westbound right-turn lane, a southbound through/right and southbound left-turn lane at the SW Scholls Ferry Road/SW Strobel Road/SW Vandermost Road intersection as part of site development. In the future, the westbound right-turn lane can be converted to a through/right lane as part of the planned widening of SW Scholls Ferry Road to five lanes.
- Stop control should be provided on the southbound approach of the SW Scholls Ferry Road/SW Strobel Road/SW Vandermost Road in accordance with applicable City and County design standards.

- West Hills should locate and maintain all future landscaping, above-ground utilities, and site signage to ensure minimum required sight lines are provided at all intersections within the Lolich/Bellairs neighborhood.

INTRODUCTION

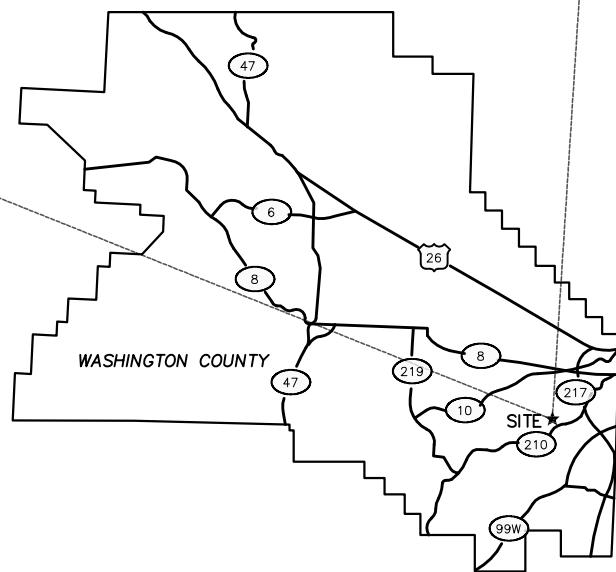
West Hills is proposing to develop up to 110 single family homes and up to 200 multifamily units on the Lolich/Bellairs property, located northeast of the SW Scholls Ferry Road/SW Strobel Road/SW Vandermost Road intersection. The existing single family home north of the proposed collector roadway may remain in-place after the new homes are constructed. Figure 1 shows the site vicinity of the proposed development and Figure 2 shows the proposed site plan.

In the near-term, access to the single family homes is proposed via SW Strobel Road north of SW Scholls Ferry Road. In the future, a new collector roadway will extend east and south from the property to SW Scholls Ferry Road, just to the west of MountainSide High School, and will serve several other area developments. At this point, the new collector will only extend to the eastern property boundary.

SCOPE OF THE REPORT

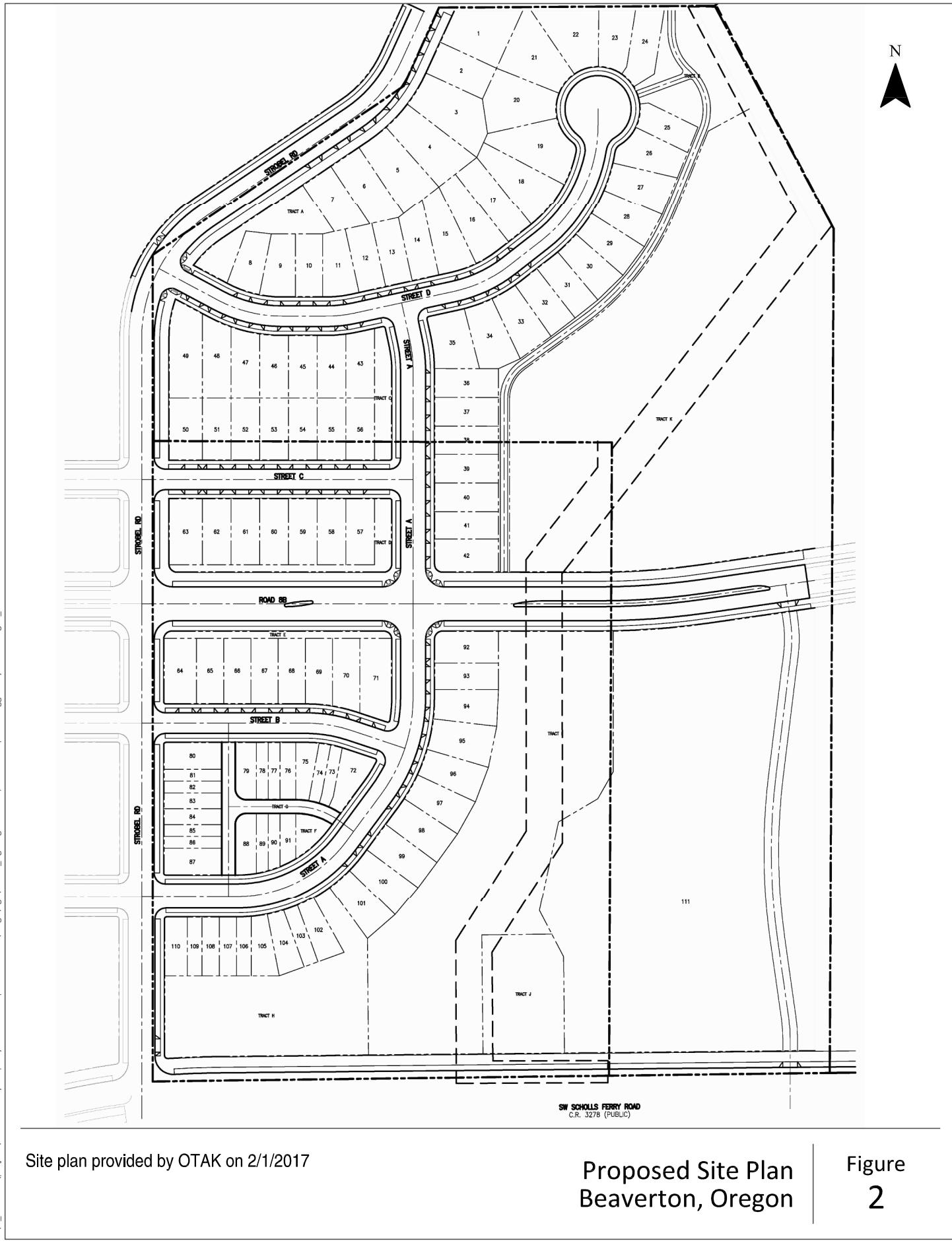
This report identifies the transportation-related impacts associated with the proposed Lolich/Bellairs Property development and was prepared in accordance with Washington County requirements for an Access Report and City of Beaverton Traffic Impact Study requirements. Per the County and City requirements (as outlined in Development Code Section 60.55.20), the study intersections include neighborhood routes, collectors, and arterials within 1,000 feet of the site, those intersections through which site-generated trips contribute five percent or more of existing volumes, as well as intersections immediately adjacent to the proposed site accesses. Accordingly, operational analyses were performed at the following study intersections during the weekday AM and PM peak periods:

- SW Scholls Ferry Road/SW Tile Flat Road;
- SW Scholls Ferry Road/SW Strobel Road/SW Vandermost Road; and
- SW Scholls Ferry Road/SW 175th Avenue/SW Roy Rogers Road.



Site Vicinity
Beaverton, Oregon

Figure
1



Site plan provided by OTAK on 2/1/2017

Proposed Site Plan
Beaverton, Oregon

Figure
2

This report evaluates the following:

- Existing land use and transportation system conditions within the site vicinity during the weekday AM and PM peak periods;
- Build-out year 2018 background traffic conditions during the weekday AM and PM peak periods, considering in-process developments and planned transportation improvements in the study area;
- Trip generation and distribution estimates for the proposed PUD;
- Build-out year 2018 total traffic conditions during the weekday AM and PM peak assuming full build-out of the proposed PUD; and
- Recommended improvements/intersection considerations.

Analysis Methodology

The intersection operational analyses presented in this report were prepared following Highway Capacity Manual 2000 (Reference 1) analysis procedures using Synchro 9 software.

Operating Standards

Intersection performance measures reported in this study include level of service (LOS), volume-to-capacity ratio (v/c), and delay. Queuing at key intersections was also assessed. Intersection operating standards adopted by the respective transportation review authorities for the facilities they operate and maintain are summarized in this section.

City of Beaverton

The City of Beaverton's Development Code sets operating standards for signalized and unsignalized intersections, found in Section 60.55.10. The standards require an average control delay of no more than 65 seconds per vehicle for signalized intersections. In addition, the volume-to-capacity (v/c) ratio for each lane group must not exceed 0.98. For two-way and all-way stop-controlled intersections, the City of Beaverton standards require an average control delay of no more than 45 seconds per vehicle. The proposed system of new streets to serve the neighborhood will be under the jurisdiction of the City.

Washington County

Washington County sets operating standards for both signalized and unsignalized intersections with a v/c no greater than 0.99 over a 60-minute period. Within the study area, Washington County maintains jurisdiction over the study intersections along SW Scholls Ferry Road.

EXISTING CONDITIONS

This section summarizes the existing characteristics of the transportation system and adjacent land uses in the vicinity of the proposed neighborhood, including an inventory of the existing multimodal transportation facilities and options, an evaluation of existing intersection operations for motor vehicles at the study intersections, and a summary of recent crash history.

Site Conditions and Adjacent Land Uses

The proposed development is located north of SW Scholls Ferry Road and to the west of the new Mountainside High School. The existing property is occupied by the Lolich Family Farm and a single family residence. The area to the south is outside of the Urban Growth Boundary whereas the areas immediately to the west and to the east are planned for future neighborhood development, as part of the South Cooper Mountain Plan. In addition, several new neighborhoods are planned for development within the River Terrace Community Plan Area.

Transportation Facilities

Access to the new neighborhood is proposed via SW Strobel Road north of SW Scholls Ferry Road. Table 1 summarizes the attributes of the arterial and collector roadways within the influence area. Figure 3 illustrates the existing lane configurations and traffic control at the study intersections.

Table 1. Street Characteristics in Site Vicinity

Street	Classification ¹	Vehicle Travel Lanes	Posted Speed (mph)	Pedestrian Facilities	Bicycle Facilities
SW 175 th Avenue/ SW Roy Rogers Road	Arterial	2- 3	45	None	Shoulder (S of Scholls)
SW Scholls Ferry Road	Arterial	4 – 5 (E of 175 th) 2 – 3 (W of 175 th)	40	Sidewalk (E of 175 th)	Bike lanes (E of 175 th) Shoulder (W of 175 th)
SW Tile Flat Road	Arterial	2	NP	None	None
SW Strobel Road/ SW Vandermost Road	Local Street	2	NP	None	None

¹Per the City of Beaverton Transportation System Plan.

²NP = Not Posted.

Pedestrian Facilities

Given the rural nature of the existing land uses, sidewalks are provided intermittently throughout the study area. Sidewalks are provided on SW Scholls Ferry Road east of SW 175th Avenue and along the frontage of the Mountainside High School. Significant pedestrian infrastructure will be provided as part of the buildup of the South Cooper Mountain and River Terrace neighborhoods.

Bicycle Facilities

Bike lanes are provided along SW Scholls Ferry Road east of SW 175th Avenue. SW 175th Avenue south of SW Scholls Ferry Road and SW Scholls Ferry Road west of SW 175th Avenue have shoulders that serve bicycles. New bike lanes will be developed as part of the buildout of the South Cooper Mountain and River Terrace neighborhoods.

Transit Facilities

The site is located near the border of the TriMet transit district. The nearest bus line is Route 92, the South Beaverton Express, which stops at the intersection of SW Scholls Ferry Road/SW Teal Boulevard/SW Horizon Boulevard (approximately 1.75 miles east of the project site). Route 92 provides weekday rush-hour service between Beaverton and the Portland City Center, including a stop at the Progress Ridge Park & Ride.

Existing Conditions Operational Analysis

Manual turning movement counts were collected at the study intersections in December 2016 when school was in session and no inclement weather conditions occurred. Traffic counts were collected for three mid-week days, in accordance with the City of Beaverton's Traffic Impact Analysis requirements for the morning (7:00 AM to 9:00 AM) and evening (4:00 PM to 6:00 PM) peak time periods. The day experiencing the highest collective volumes during the peak hours was selected for the operational analysis (i.e., Tuesday, December 13th for the AM and Tuesday, December 6th for the PM peak). Figure 3 summarizes the traffic volumes at the study intersections during both peak hours. *Appendix "A" contains the traffic count worksheets.*

Table 2 summarizes the operational analysis for the study intersections during the weekday AM and PM peak hours. Each of the study intersections currently meets County standards (i.e., v/c < 0.99 during 60-minute period). All of the intersections meet City standards during the peak 15 minute period, with the exception of the high delays experienced at the southbound SW Strobel Road approach to the SW Scholls Ferry Road intersection during the weekday PM peak hour. As shown in Figure 3, only three vehicles make this movement today and per Table 2, the v/c ratio for this movement is 0.05. As will be discussed, West Hills will be providing improvements to this intersection in conjunction with site development. *Appendix "B" includes the operations analysis worksheets for the Existing Conditions analysis.*

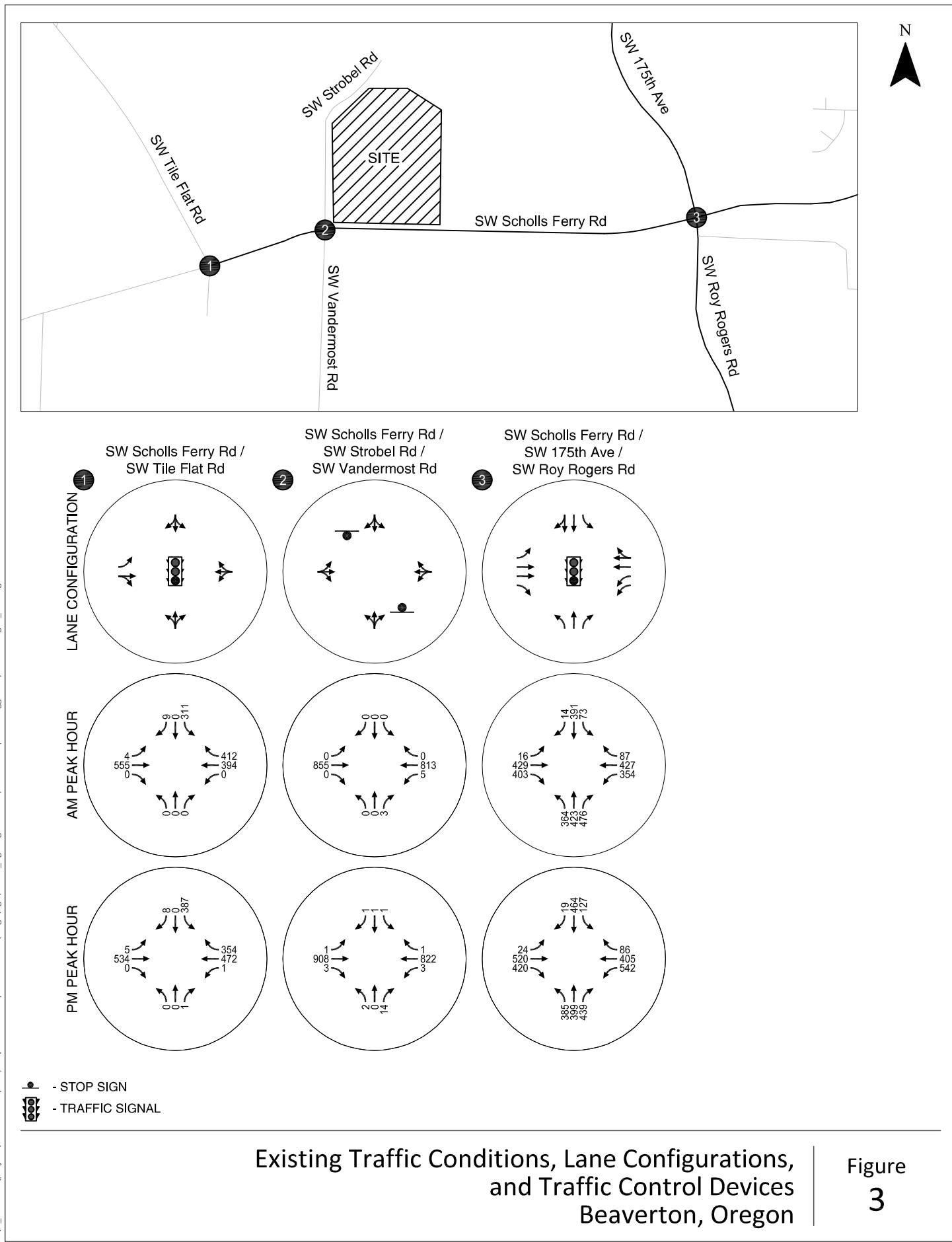


Table 2. Existing Intersection Operations

Study Intersection		60-Minute Results			Peak 15-Minute Results		
		V/C	Delay (sec)	LOS	V/C	Delay (sec)	LOS
<i>Weekday AM Peak Hour</i>							
1	SW Scholls Ferry Road/ SW Tile Flat Road	0.82	17.6	B	0.91	23.4	C
2	SW Scholls Ferry Road/ SW Strobel Road/SW Vandermost Road	0.01 (NB)	16.6 (NB)	C	0.01 (NB)	19.1 (NB)	C
3	SW Scholls Ferry Road/ SW 175 th Avenue/SW Roy Rogers Road	0.74	30.7	C	0.82	34.6	C
<i>Weekday PM Peak Hour</i>							
1	SW Scholls Ferry Road/ SW Tile Flat Road	0.82	25.7	C	0.91	30.6	C
2	SW Scholls Ferry Road/ SW Strobel Road/SW Vandermost Road	0.04 (SB)	> 50 (SB)	F	0.05 (SB)	> 50 (SB)	F
3	SW Scholls Ferry Road/ SW 175 th Avenue/SW Roy Rogers Road	0.80	37.3	D	0.85	44.1	D

Traffic Safety

Washington County maintains a Safety Priority Index System (SPIS) list to identify existing hazardous intersections for potential safety improvements. Intersections are included in the SPIS list if they have three or more crashes or if they have one or more severe injury or fatal crashes within three consecutive years. The SW Roy Rogers Road/SW Scholls Ferry Road/SW 175th Avenue intersection is ranked #99 on the most recent Washington County SPIS list (2012-2014, Reference 2). The SPIS list notes that there was a Major Streets Transportation Improvement Program (MSTIP) project completed at the intersection in 2014 and that a number of planned developments are anticipated in the vicinity.

In addition to reviewing the Washington County SPIS list, the crash history of each study intersection was reviewed in an effort to identify potential intersection safety issues. Crash data for the study intersections was obtained from the Oregon Department of Transportation (ODOT) for the five-year period from January 1, 2010 through December 31, 2014. Table 3 illustrates the crashes reported at the study intersections. Note that a traffic signal was installed at the SW Scholls Ferry Road/SW Tile Flat Road intersection in 2014. *Appendix "C" contains the ODOT crash data.*

Table 3. Intersection Crash History (January 1, 2010 through December 31, 2014)

Location	Collision Type						Severity		Total Crashes	Crash Rate ⁴	Statewide 90 th - percentile Crash Rate ⁴	Observed Crash Rate > Statewide 90 th - percentile?
	Turning	Fixed Object	Rear End	Sideswipe	Angle	PDO ¹	Injury					
SW Scholls Ferry Road/ SW Tile Flat Road ²	6	0	0	0	0	3	3	6	0.19	0.48		No
SW Scholls Ferry Road/ SW Strobel Road/ SW Vandermost Road	1	0	0	0	0	0	1	1	0.03	1.08		No
SW Scholls Ferry Road/ SW 175 th Avenue/ SW Roy Rogers Road ³	6	3	17	1	2	18	11	29	0.42	0.86		No

¹PDO – Property damage only

²Traffic signal installed in 2014.

³ Reported crashes occurred prior to completion of MSTIP project improvements.

⁴Crash rate per million entering vehicles

As shown in Table 3, the observed crash rate at each intersection was compared to the statewide 90th-percentile crash rate for similar intersection types (rural/urban, signalized/stop-controlled, 3-leg/4-leg), consistent with the ODOT Analysis Procedures Manual (Reference 3). As shown, none of the observed crash rates at the study intersections exceed the statewide 90th-percentile crash rates. As a result, no safety-related mitigation measures are recommended as part of the development of the PUD.

TRAFFIC IMPACT ANALYSIS

The traffic impact analysis identifies how the study area's transportation system will operate in the build-out year 2018 when the single family homes are expected to be occupied. As discussed previously, although the multifamily homes are not proposed for construction at this time, we also assumed occupancy of these units in 2018.

This section of the report addresses the following elements:

- Build-out year 2018 background traffic conditions during the weekday AM and PM peak periods, considering in-process developments and planned transportation improvements in the study area;
- Trip generation and distribution estimates for the proposed PUD;
- Build-out year 2018 total traffic conditions during the weekday AM and PM peak assuming full build-out of the proposed PUD; and
- Recommended improvements/intersection considerations.

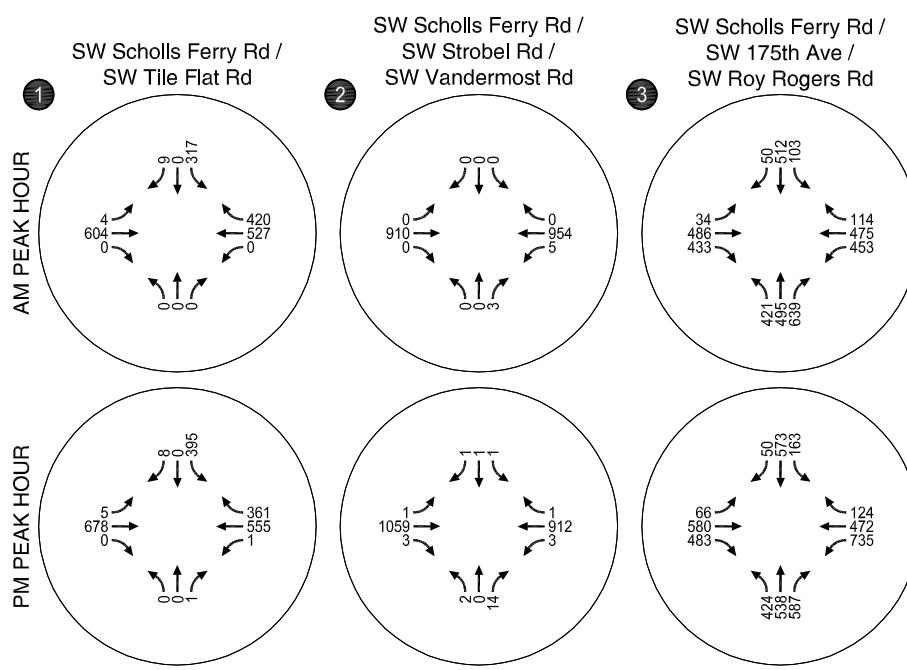
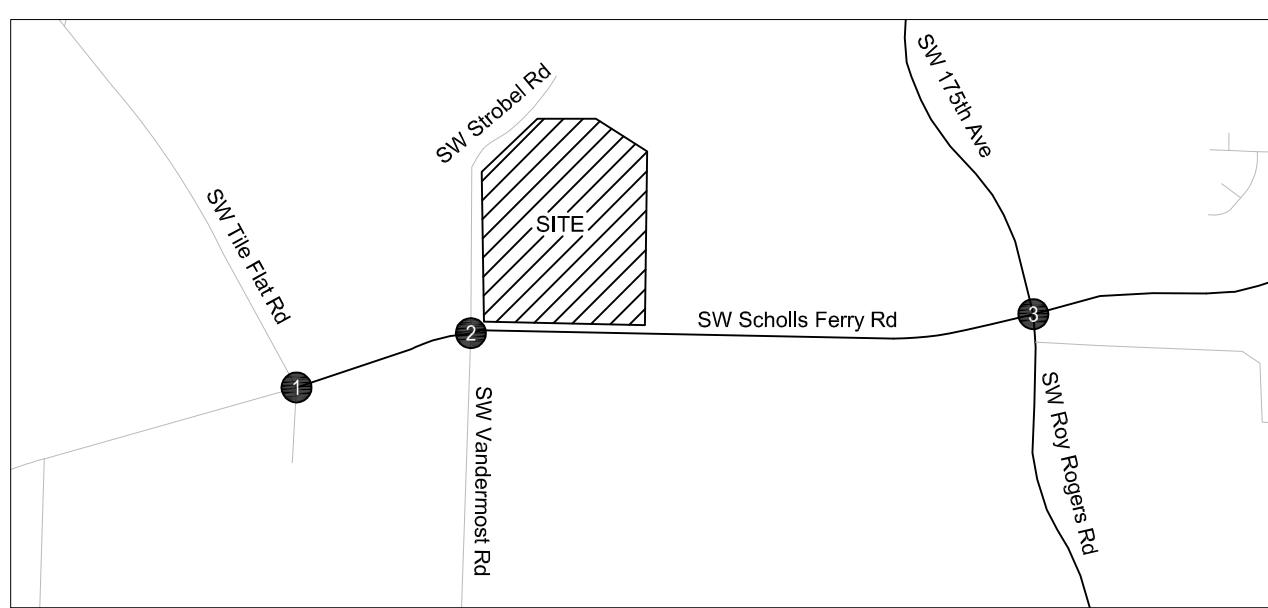
2018 Background Operational Analysis

Background traffic volumes include changes in volumes due to added trips from new development in the vicinity as well as general regional growth. The year 2018 analyses include a background annual growth rate of two percent at each study intersection, as well as the following in-process developments identified by City and County staff:

- South Cooper Mountain Heights;
- River Terrace Northwest;
- River Terrace East;
- West River Terrace;
- Roshak Ridge;
- South River Terrace; and,
- Bull Mountain Dickson.

Figure 4 summarizes the background traffic volumes at the study intersections during both peak hours. Table 4 summarizes the operational analysis for the study intersections during the weekday AM and PM peak hours.

As shown in Table 4, each of the study intersections is forecast to meet County standards (i.e., v/c < 0.99 during 60-minute period) under the background conditions. All of the intersections are also forecast to meet City standards during the peak 15 minute period, with two exceptions. During the weekday PM peak hour, the southbound SW Strobel Road approach to the SW Scholls Ferry Road intersection is forecast to continue to function with high delays but an acceptable v/c ratio. In addition, the SW Scholls Ferry/SW 175th Avenue/SW Roy Rogers Road is forecast to operate at LOS "E" and a v/c of 0.99 during the peak 15-minute time period. It should be noted that several ongoing land use reviews have recognized the near-capacity conditions at this intersection and no additional improvements are suggested at this time beyond constructed as part of the MSTIP project. *Appendix "D" includes the operations analysis worksheets for the 2018 Background Conditions analysis, as well as a summary of the in-process traffic volumes.*



Year 2018 Background Traffic Conditions
Weekday AM and PM Peak Hours
Beaverton, Oregon

Figure
4

Table 4. Background Intersection Operations

Study Intersection		60-Minute Results			Peak 15-Minute Results		
		V/C	Delay (sec)	LOS	V/C	Delay (sec)	LOS
<i>Weekday AM Peak Hour</i>							
1	SW Scholls Ferry Road/ SW Tile Flat Road	0.86	21.1	C	0.96	28.7	C
2	SW Scholls Ferry Road/ SW Strobel Road/SW Vandermost Road	0.01 (NB)	17.6 (NB)	C	0.01 (NB)	20.1 (SB)	C
3	SW Scholls Ferry Road/ SW 175 th Avenue/SW Roy Rogers Road	0.83	39.3	D	0.91	47.7	D
<i>Weekday PM Peak Hour</i>							
1	SW Scholls Ferry Road/ SW Tile Flat Road	0.86	27.9	C	0.94	36.4	D
2	SW Scholls Ferry Road/ SW Strobel Road/SW Vandermost Road	0.07 (SB)	> 50 (SB)	F	0.11 (SB)	> 50 (SB)	F
3	SW Scholls Ferry Road/ SW 175 th Avenue/SW Roy Rogers Road	0.95	53.7	D	0.99	59.4	E

Trip Generation Estimate

Trip generation estimates for the PUD were developed based on the standard reference manual published by the Institute of Transportation Engineers, *Trip Generation*, 9th Edition (Reference 4). The estimated trip generation is shown in Table 5. As mentioned previously, the existing family home north of the proposed collector may remain in-place and as such is accounted for in the existing and background volumes.

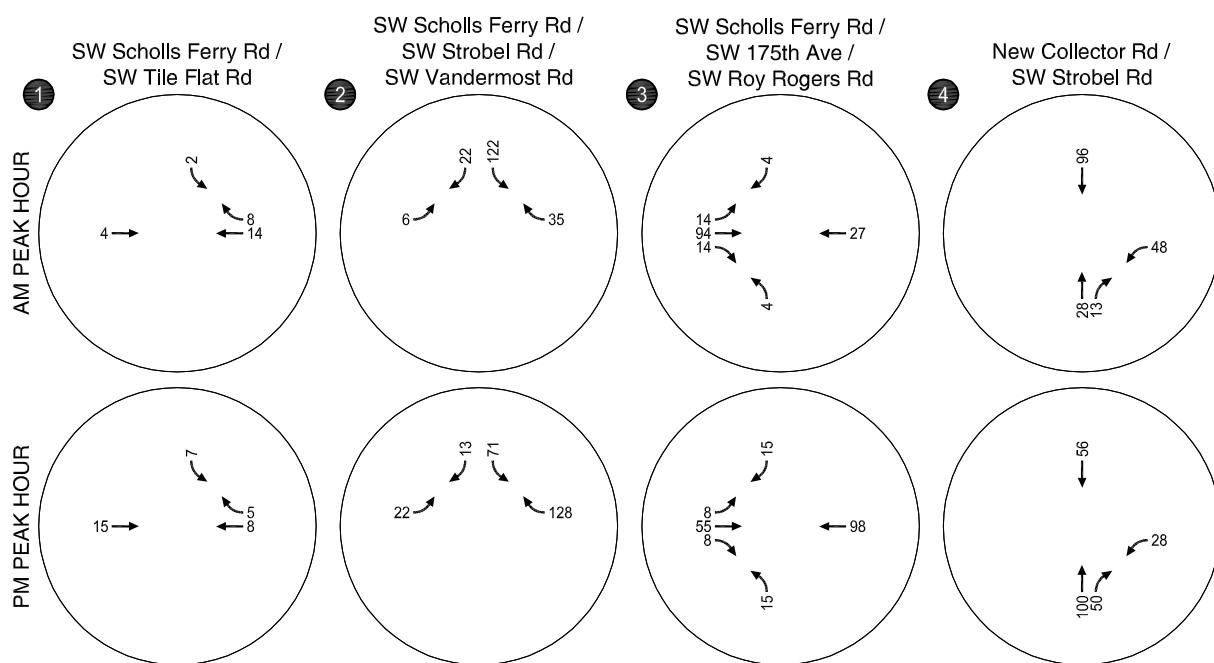
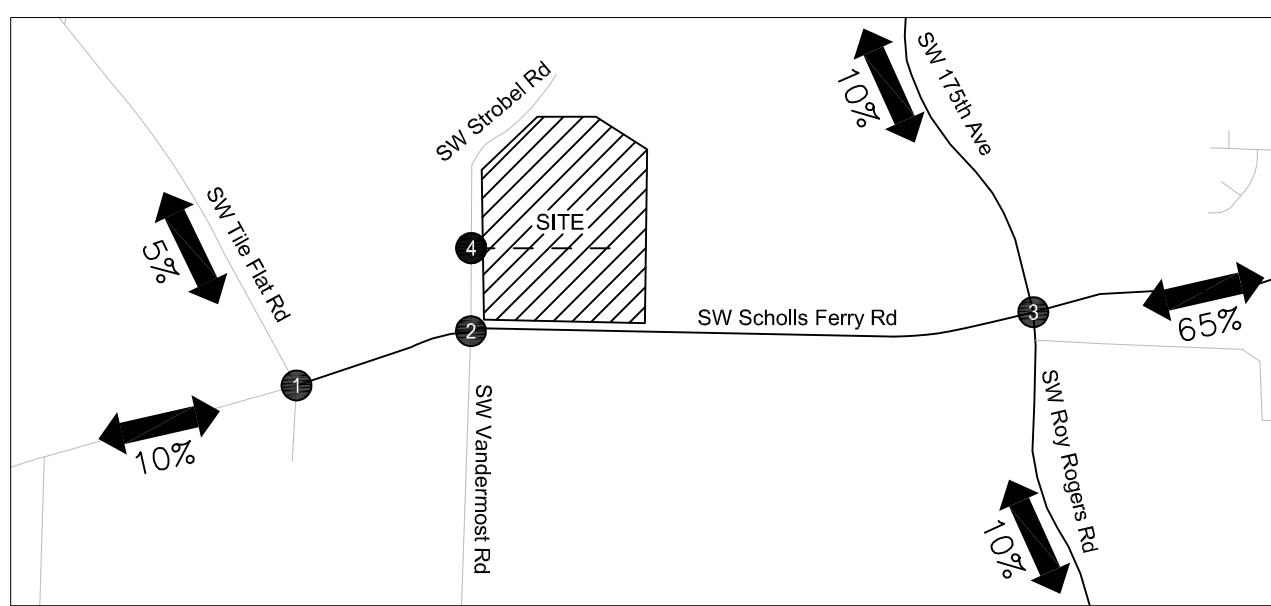
Table 5. Estimated Trip Generation

	ITE Code	Dwelling Units	Total Daily Trips	Weekday AM Peak Hour			Weekday PM Peak Hour		
				Total Trips	In	Out	Total Trips	In	Out
Single Family	210	110	1,048	83	21	62	110	69	41
Multi-Family	220	200	1,330	102	20	82	124	81	43
Total		310	2,378	185	41	144	234	150	84

Trip Distribution and Assignment

Figure 5 illustrates the estimated trip distribution pattern. This distribution pattern is similar to that used on other ongoing land use cases in the study vicinity.

The estimated site-generated trips were assigned to the network by distributing the trips shown in Table 5 according to the trip distribution pattern. Figure 5 shows the site-generated trips that are expected to use the study intersections during the peak hour periods.



**Site-Generated Trips
Weekday AM and PM Peak Hours
Beaverton, Oregon**

Figure
5

2018 Total Traffic Operational Analysis

Total traffic volumes include the site-generated trips in addition to the 2018 background traffic volumes. Figure 6 summarizes the total traffic volumes at the study intersections during both peak hours. Table 6 summarizes the operational analysis for the study intersections during the weekday AM and PM peak hours. *It should be noted that the operational results for the SW Scholls Ferry Road/SW Strobel Road/SW Vandermost Road assume the construction of a separate southbound left-turn lane, eastbound left-turn lane, westbound right-turn lane, and westbound left-turn lane as part of site development at this intersection.*

Table 6. Total Intersection Operations

Study Intersection		60-Minute Results			Peak 15-Minute Results		
		V/C	Delay (sec)	LOS	V/C	Delay (sec)	LOS
<i>Weekday AM Peak Hour</i>							
1	SW Scholls Ferry Road/ SW Tile Flat Road	0.88	21.9	C	0.98	30.6	C
2	SW Scholls Ferry Road/ SW Strobel Road/SW Vandermost Road	0.57 (SBLT)	42.5 (SBLT)	E	0.77 (SBLT)	> 50 (SBLT)	F
3	SW Scholls Ferry Road/ SW 175 th Avenue/SW Roy Rogers Road	0.87	41.8	D	0.95	51.2	D
4	SW Strobel Road/ New Collector	0.06 (WBLT)	9.4 (WBLT)	A	0.06 (WBLT)	9.5 (WBLT)	A
<i>Weekday PM Peak Hour</i>							
1	SW Scholls Ferry Road/ SW Tile Flat Road	0.87	29.1	C	0.95	38.7	D
2	SW Scholls Ferry Road/ SW Strobel Road/SW Vandermost Road	0.43 (SBLT)	42.2 (SBLT)	E	0.57 (SBLT)	> 50 (SBLT)	F
3	SW Scholls Ferry Road/ SW 175 th Avenue/SW Roy Rogers Road	0.97	56.8	E	1.02	62.9	E
4	SW Strobel Road/ New Collector	0.03 (WBLT)	9.6 (WBLT)	A	0.04 (WBLT)	9.7 (WBLT)	A

As shown in Table 6, each of the study intersections is forecast to meet County standards (i.e., v/c < 0.99 during 60-minute period) assuming development of the PUD. Similar to the 2018 background conditions, all of the intersections meet City standards during the peak 15 minute period, with two exceptions. During the weekday PM peak hour, the southbound SW Strobel Road approach to the SW Scholls Ferry Road intersection is forecast to continue to function with high delays but an acceptable v/c ratio. In addition, the SW Scholls Ferry/SW 175th Avenue/SW Roy Rogers Road is forecast to operate at LOS "E" and a v/c of 1.02 during the peak 15-minute time period. As previously noted, several ongoing land use reviews have recognized the near-capacity conditions at this intersection and no additional improvements are suggested at this time beyond those constructed as part of the MSTIP project. *Appendix "E" includes the year 2018 total traffic operations worksheets.*

SW Scholls Ferry Road/SW Strobel Road Turn Lane Warrant Analysis

As noted above, the 2018 total traffic operations results are reported assuming that West Hills construct the following improvements as part of site development:

- Eastbound left-turn lane with 100 feet of storage;
- Westbound left-turn lane with 100 feet of storage;
- Westbound right-turn lane with 100 feet of storage; and,
- Southbound left-turn lane with 125 feet of storage.

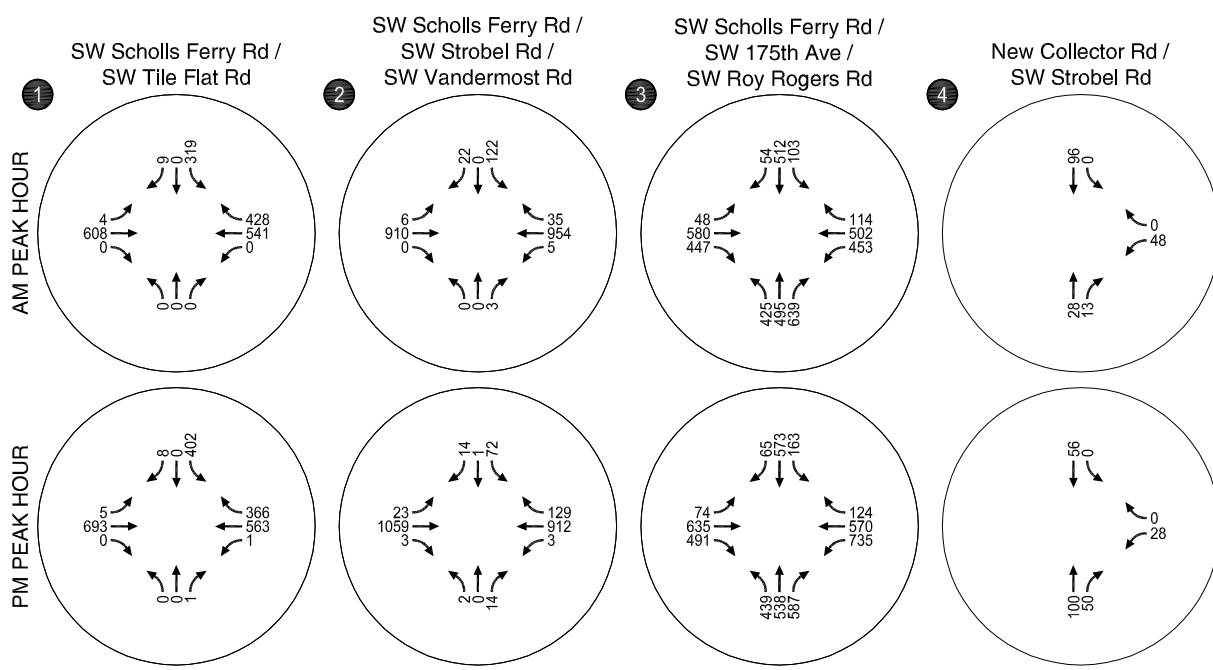
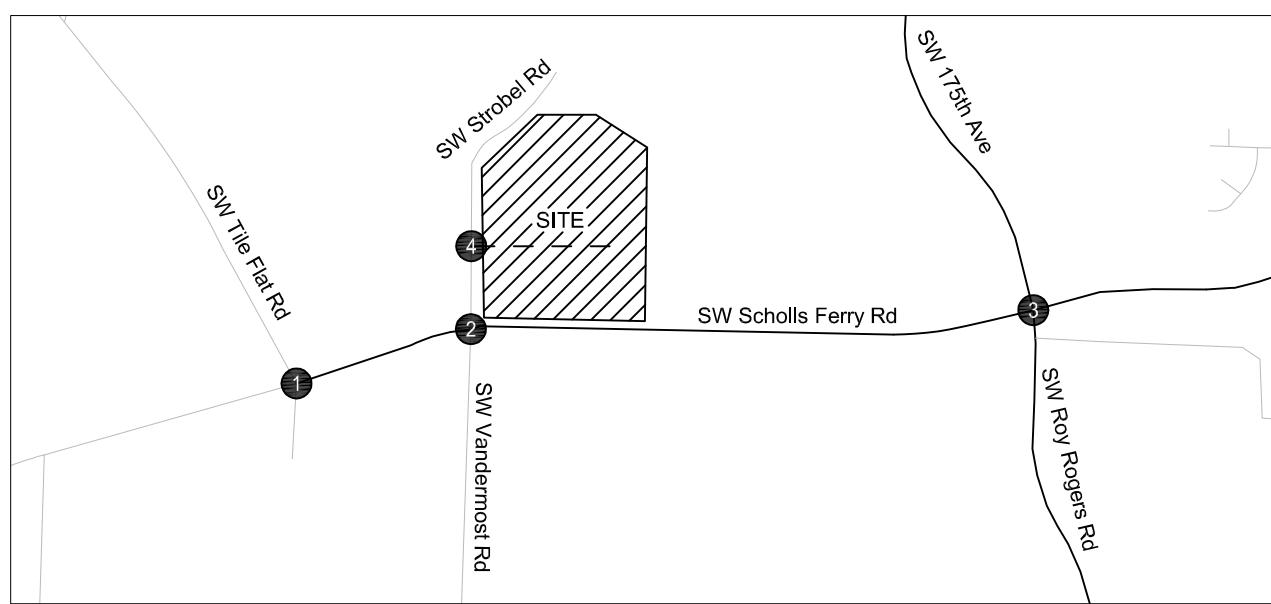
Stop control should be provided on the southbound approach to the intersection in accordance with applicable City and County design standards. Finally, in the future, the westbound right-turn lane can be converted to a through/right lane as part of the planned widening of SW Scholls Ferry Road to five lanes.

SW Scholls Ferry Road/SW Strobel Road Queueing Warrant Analysis

Table 7 identifies the projected 95th percentile queueing at the SW Scholls Ferry Road/SW Strobel Road intersection assuming full occupancy of the single family and multifamily homes proposed within the PUD and the added turn lanes to be constructed by West Hills. Note that these results could be expected to improve over time as alternative site access is created to the east with other off-site development.

Table 7. 95th Percentile Queues at SW Scholls Ferry Road/SW Strobel Road for peak 15-minute analysis

Movement	95th-percentile Queue (ft)	
	Total AM	Total PM
Eastbound Left	<25	<25
Southbound Left	125	75
Southbound Through/Right	<25	<25
Westbound Right	<25	<25
Northbound Left/Through/Right	<25	<25



Year 2018 Total Traffic Conditions
Weekday AM and PM Peak Hours
Beaverton, Oregon

Figure
6

SW Strobel Road/Future Collector Intersection

Based on conversations with City staff, we evaluated the need for a southbound left-turn lane on SW Strobel Road at the future collector street intersection. Until such a time that the collector is extended southeast to SW Scholls Ferry Road and additional properties develop both east and west of the PUD, this improvement will not be needed. However, West Hills will construct SW Strobel Road to allow for the provision of a southbound left-turn lane at this location in the future.

Intersection Sight Distance

West Hills should locate and maintain all future landscaping, above-ground utilities, and site signage to ensure they provide minimum required sight lines are provided at all intersections within the Lolich/Bellairs neighborhood.

OTAK will address sight distance at the SW Strobel/SW Scholls Ferry Road intersection under separate cover with the identified improvements in-place.

RECOMMENDATIONS

Based on the analysis herein, the following recommendations are associated with the proposed PUD:

- Per prior traffic impact studies approved for River Terrace and South Cooper Mountain, Washington County should continue to monitor the SW Scholls Ferry Road/SW 175th Avenue/SW Roy Rogers Road intersection to determine if, and when, additional modifications are needed beyond those currently planned for construction.
- West Hills should improve the SW Scholls Ferry Road/SW Strobel Road/Vandermost Road intersection as part of site development to provide:
 - an eastbound left-turn lane (with at least 100 feet of storage);
 - a westbound left-turn lane (with at least 100 feet of storage);
 - a westbound right-turn lane (with at least 100 feet of storage), which can be converted in the future to a through/right lane as part of the planned widening of SW Scholls Ferry Road to five lanes;
 - a southbound through/right and southbound left-turn lane (with at least 125 feet of storage), and
 - stop control on the southbound approach in accordance with applicable City and County design standards.
- West Hills should locate and maintain all future landscaping, above-ground utilities, and site signage to ensure minimum required sight lines are provided at all intersections within the Lolich/Bellairs neighborhood.

Please contact us at (503) 228-5230 if you have any questions regarding this study or the findings and recommendations presented.

REFERENCES

1. Transportation Research Board. *2000 Highway Capacity Manual*. 2000.
2. Washington County, Oregon. *Safety Priority Index System (SPIS)*. 2012-2014.
3. Oregon Department of Transportation. *Analysis Procedures Manual Version 2*. 2016.
4. Institute of Transportation Engineers. *Trip Generation, 9th Edition*. 2012.

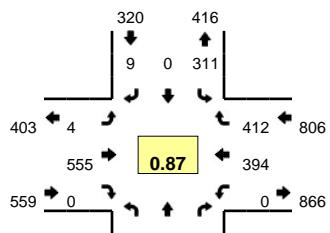
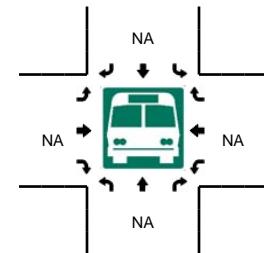
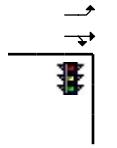
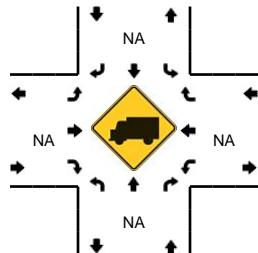
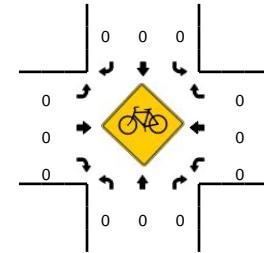
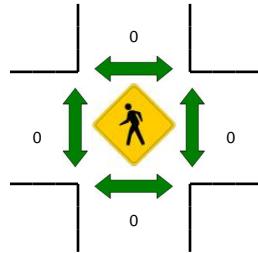
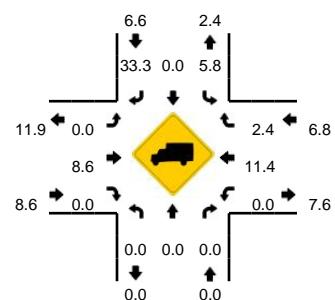
APPENDICES

- A. Traffic Counts
- B. Year 2016 Existing Traffic Conditions Worksheets
- C. ODOT Crash Data
- D. Year 2018 Background Traffic Conditions Worksheets
- E. Year 2018 Total Traffic Conditions Worksheets

Appendix A Traffic Counts

Type of peak hour being reported: User-Defined

Method for determining peak hour: Total Entering Volume

LOCATION: SW Tile Flat Rd -- SW Scholls Ferry Rd
CITY/STATE: Beaverton, OR
QC JOB #: 14075201**DATE:** Tue, Dec 13 2016
Peak-Hour: 7:10 AM -- 8:10 AM
Peak 15-Min: 7:30 AM -- 7:45 AM


5-Min Count Period Beginning At	SW Tile Flat Rd (Northbound)				SW Tile Flat Rd (Southbound)				SW Scholls Ferry Rd (Eastbound)				SW Scholls Ferry Rd (Westbound)				Total	Hourly Totals
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
7:00 AM	0	0	0	0	24	0	0	0	2	50	0	0	0	28	13	0	117	
7:05 AM	0	0	0	0	20	0	0	0	0	49	0	0	0	18	24	0	111	
7:10 AM	0	0	0	0	31	0	0	0	0	39	0	0	0	36	34	0	140	
7:15 AM	0	0	0	0	24	0	0	0	1	54	0	0	0	26	34	0	139	
7:20 AM	0	0	0	0	23	0	1	0	0	49	0	0	0	35	34	0	142	
7:25 AM	0	0	0	0	33	0	0	0	0	51	0	0	0	34	34	0	152	
7:30 AM	0	0	0	0	29	0	2	0	0	48	0	0	0	42	32	0	153	
7:35 AM	0	0	0	0	29	0	1	0	0	48	0	0	0	39	35	0	152	
7:40 AM	0	0	0	0	31	0	1	0	1	55	0	0	0	41	49	0	178	
7:45 AM	0	0	0	0	26	0	0	0	0	39	0	0	0	28	34	0	127	
7:50 AM	0	0	0	0	12	0	1	0	0	52	0	0	0	27	34	0	126	
7:55 AM	0	0	0	0	25	0	1	0	1	38	0	0	0	27	29	0	121	1658
8:00 AM	0	0	0	0	26	0	2	0	0	32	0	0	0	28	36	0	124	1665
8:05 AM	0	0	0	0	22	0	0	0	1	50	0	0	0	31	27	0	131	1685
8:10 AM	0	0	0	0	17	0	0	0	0	38	0	0	0	27	25	0	107	1652
8:15 AM	0	0	0	0	27	0	0	0	0	40	0	0	0	27	28	0	122	1635
8:20 AM	0	0	0	0	24	0	0	0	0	50	0	0	0	34	35	0	143	1636
8:25 AM	0	0	0	0	19	0	1	0	0	37	0	0	0	36	20	0	113	1597
8:30 AM	0	0	0	0	27	0	0	0	0	26	0	0	0	26	24	0	103	1547
8:35 AM	0	0	0	0	17	0	0	0	1	45	0	0	0	29	16	0	108	1503
8:40 AM	0	0	0	0	33	0	1	0	0	38	0	0	0	30	24	0	126	1451
8:45 AM	0	0	0	0	16	0	0	0	1	24	0	0	0	25	25	0	91	1415
8:50 AM	0	0	0	0	21	0	1	0	0	25	0	0	0	25	15	0	87	1376
8:55 AM	0	0	0	0	11	0	0	0	0	29	0	0	0	29	19	0	88	1343
Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound					
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Total	
All Vehicles	0	0	0	0	356	0	16	0	4	604	0	0	0	488	464	0		1932
Heavy Trucks	0	0	0	0	16	0	8	0	0	48	0	0	0	48	12	0		132
Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		0
Bicycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		0
Railroad	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		0
Stopped Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		0

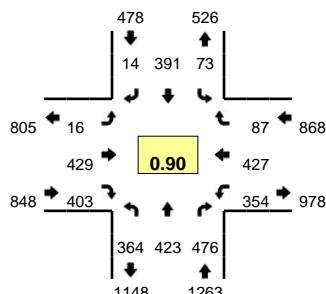
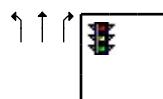
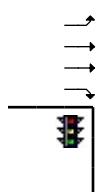
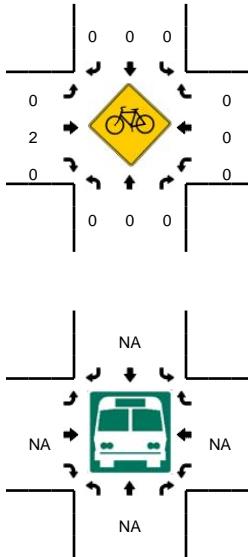
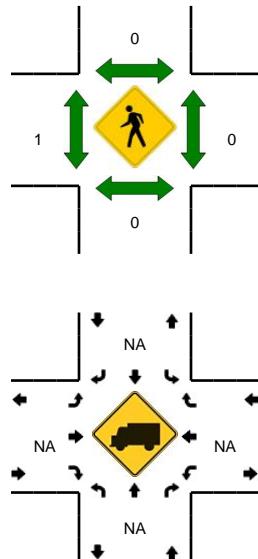
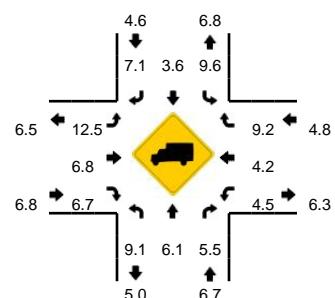
Comments:

Report generated on 1/31/2017 11:21 AM

SOURCE: Quality Counts, LLC (<http://www.qualitycounts.net>) 1-877-580-2212

Type of peak hour being reported: User-Defined

Method for determining peak hour: Total Entering Volume

LOCATION: SW Roy Rogers Rd -- SW Scholls Ferry Rd
CITY/STATE: Sherwood, OR
QC JOB #: 14075207**DATE:** Tue, Dec 13 2016
Peak-Hour: 7:10 AM -- 8:10 AM
Peak 15-Min: 7:30 AM -- 7:45 AM


5-Min Count Period Beginning At	SW Roy Rogers Rd (Northbound)				SW Roy Rogers Rd (Southbound)				SW Scholls Ferry Rd (Eastbound)				SW Scholls Ferry Rd (Westbound)				Total	Hourly Totals
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
7:00 AM	17	41	35	0	3	20	1	0	1	33	34	0	28	25	7	0	245	
7:05 AM	28	25	40	0	8	27	1	0	0	24	36	0	15	15	10	0	229	
7:10 AM	38	34	34	0	10	14	2	0	0	34	26	0	26	34	7	0	259	
7:15 AM	20	30	42	0	2	36	1	0	2	48	46	0	39	40	9	0	315	
7:20 AM	35	41	38	0	10	41	1	0	3	29	27	0	23	37	2	0	287	
7:25 AM	29	22	36	0	3	30	1	0	1	38	41	0	32	41	5	0	279	
7:30 AM	34	46	45	0	2	46	0	0	1	41	38	0	26	41	11	0	331	
7:35 AM	25	33	38	0	2	28	3	0	1	40	47	0	45	59	7	0	328	
7:40 AM	38	32	43	0	11	48	1	0	1	23	39	0	27	27	6	0	296	
7:45 AM	25	37	51	0	5	24	1	0	3	40	39	0	33	37	6	0	301	
7:50 AM	27	56	37	0	5	40	1	0	0	28	26	0	33	32	11	0	296	
7:55 AM	28	29	32	0	11	17	2	0	2	36	25	0	12	27	6	0	227	3393
8:00 AM	30	30	43	0	5	37	1	0	2	33	30	0	29	32	9	0	281	3429
8:05 AM	35	33	37	0	7	30	0	0	0	39	19	0	29	20	8	0	257	3457
8:10 AM	11	32	50	0	8	21	3	0	0	42	15	0	28	33	3	0	246	3444
8:15 AM	28	43	33	0	1	31	1	0	1	34	30	0	10	22	5	0	239	3368
8:20 AM	27	28	43	0	5	27	2	0	1	41	36	0	24	42	3	0	279	3360
8:25 AM	30	24	28	0	4	28	1	0	5	24	26	0	22	25	4	0	221	3302
8:30 AM	23	23	30	0	8	22	0	0	0	27	30	0	18	28	8	0	217	3188
8:35 AM	20	22	34	0	4	24	0	0	1	44	15	0	30	25	5	0	224	3084
8:40 AM	30	25	38	0	4	25	1	0	0	30	31	0	25	18	5	0	232	3020
8:45 AM	27	27	40	0	2	16	2	0	1	32	25	0	25	23	4	0	224	2943
8:50 AM	13	19	28	0	7	22	1	0	1	17	15	0	29	28	2	0	182	2829
8:55 AM	19	13	33	0	8	15	2	0	2	16	18	0	15	19	8	0	168	2770
Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound				Total	
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Total	
All Vehicles	388	444	504	0	60	488	16	0	12	416	496	0	392	508	96	0	3820	
Heavy Trucks	36	24	32	0	0	32	4	0	4	24	28	0	20	28	8	0	240	
Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Bicycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Railroad																		
Stopped Buses																		

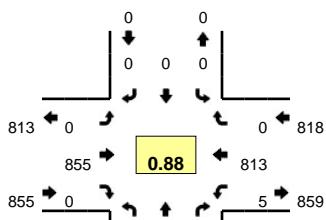
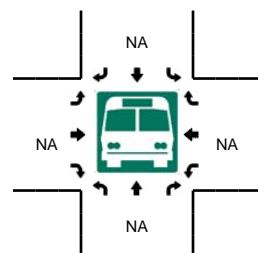
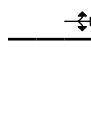
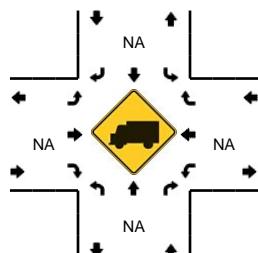
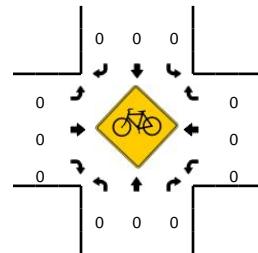
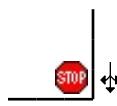
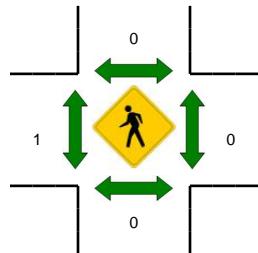
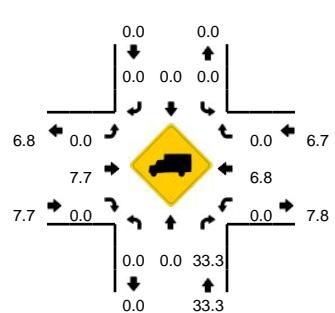
Comments:

Report generated on 1/31/2017 11:21 AM

SOURCE: Quality Counts, LLC (<http://www.qualitycounts.net>) 1-877-580-2212

Type of peak hour being reported: User-Defined

Method for determining peak hour: Total Entering Volume

LOCATION: Strobel Rd/SW Vandermost Rd -- SW Scholls Ferry Rd
CITY/STATE: Beaverton, OR
QC JOB #: 14075213**DATE:** Tue, Dec 13 2016
Peak-Hour: 7:10 AM -- 8:10 AM
Peak 15-Min: 7:30 AM -- 7:45 AM


5-Min Count Period Beginning At	Strobel Rd/SW Vandermost Rd (Northbound)				Strobel Rd/SW Vandermost Rd (Southbound)				SW Scholls Ferry Rd (Eastbound)				SW Scholls Ferry Rd (Westbound)				Total	Hourly Totals
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
7:00 AM	0	0	1	0	0	0	0	0	0	73	0	0	0	0	48	0	0	122
7:05 AM	0	0	0	0	0	0	0	0	0	63	0	0	0	0	33	0	0	96
7:10 AM	0	0	0	0	0	0	0	0	0	72	0	0	0	0	74	0	0	146
7:15 AM	0	0	0	0	0	0	0	0	0	79	0	0	0	0	58	0	0	137
7:20 AM	0	0	0	0	0	0	0	0	0	70	0	0	0	2	74	0	0	146
7:25 AM	0	0	0	0	0	0	0	0	0	81	0	0	0	0	62	0	0	143
7:30 AM	0	0	1	0	0	0	0	0	0	74	0	0	0	0	79	0	0	154
7:35 AM	0	0	0	0	0	0	0	0	0	76	0	0	0	0	88	0	0	164
7:40 AM	0	0	0	0	0	0	0	0	0	88	0	0	1	71	0	0	0	160
7:45 AM	0	0	1	0	0	0	0	0	0	58	0	0	0	0	63	0	0	122
7:50 AM	0	0	0	0	0	0	0	0	0	70	0	0	1	64	0	0	0	135
7:55 AM	0	0	1	0	0	0	0	0	0	62	0	0	0	0	53	0	0	116
8:00 AM	0	0	0	0	0	0	0	0	0	58	0	0	0	0	71	0	1	130
8:05 AM	0	0	0	0	0	0	0	0	0	67	0	0	0	0	56	0	0	123
8:10 AM	0	0	0	0	0	0	0	0	0	58	0	0	0	0	49	0	0	107
8:15 AM	0	0	0	0	0	0	0	0	0	69	1	0	0	0	53	0	0	123
8:20 AM	0	0	0	0	0	0	0	0	0	71	0	0	0	0	69	0	0	140
8:25 AM	0	0	1	0	0	0	0	0	0	59	0	0	0	0	62	0	0	122
8:30 AM	0	0	1	0	0	0	0	0	0	50	0	0	1	48	0	0	100	1596
8:35 AM	0	0	0	0	0	0	0	0	0	62	0	0	0	0	48	0	0	110
8:40 AM	0	0	0	0	0	0	0	0	0	71	0	0	0	0	47	0	0	1488
8:45 AM	0	0	0	0	0	0	0	0	2	39	0	0	1	55	0	0	0	118
8:50 AM	0	0	1	0	0	0	0	0	0	38	0	0	0	0	41	0	0	97
8:55 AM	1	0	0	0	0	0	0	0	0	42	0	0	0	0	44	0	0	87
Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound				Total	
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
All Vehicles	0	0	4	0	0	0	0	0	0	952	0	0	4	952	0	0	1912	
Heavy Trucks	0	0	0	0	0	0	0	0	0	64	0	0	0	60	0	0	124	
Pedestrians	0	0	0	0	0	0	0	0	0	4	0	0	0	0	0	0	4	
Bicycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Railroad	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Stopped Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	

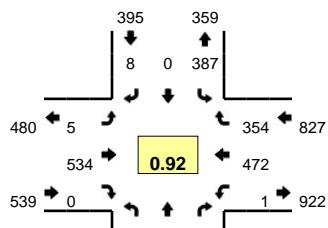
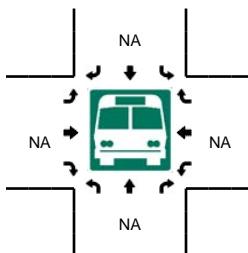
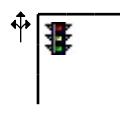
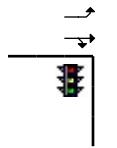
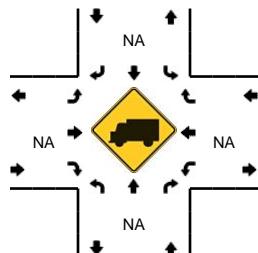
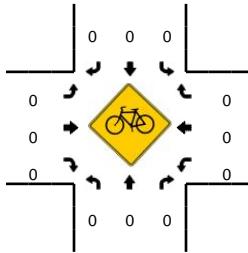
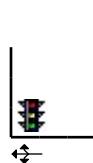
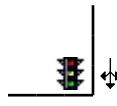
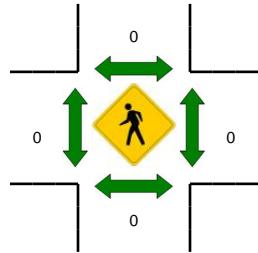
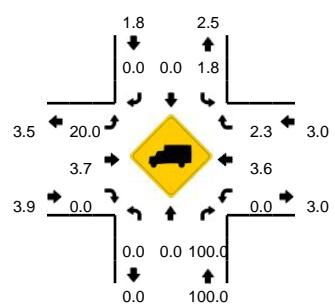
Comments:

Report generated on 1/31/2017 11:21 AM

SOURCE: Quality Counts, LLC (<http://www.qualitycounts.net>) 1-877-580-2212

Type of peak hour being reported: User-Defined

Method for determining peak hour: Total Entering Volume

LOCATION: SW Tile Flat Rd -- SW Scholls Ferry Rd
CITY/STATE: Beaverton, OR
QC JOB #: 14075204**DATE:** Tue, Dec 06 2016
Peak-Hour: 4:45 PM -- 5:45 PM
Peak 15-Min: 4:50 PM -- 5:05 PM


5-Min Count Period Beginning At	SW Tile Flat Rd (Northbound)				SW Tile Flat Rd (Southbound)				SW Scholls Ferry Rd (Eastbound)				SW Scholls Ferry Rd (Westbound)				Total	Hourly Totals
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
4:00 PM	0	0	0	0	28	0	0	0	0	18	0	0	0	43	21	0	110	
4:05 PM	0	0	0	0	28	0	1	0	1	40	0	0	0	37	25	0	132	
4:10 PM	0	0	0	0	23	0	2	0	1	33	0	0	0	30	36	0	125	
4:15 PM	0	0	0	0	11	0	0	0	0	39	0	0	0	44	24	0	118	
4:20 PM	0	0	1	0	35	0	0	0	0	34	0	0	1	33	23	0	127	
4:25 PM	0	0	0	0	33	0	0	0	0	28	0	0	0	42	27	0	130	
4:30 PM	0	0	0	0	32	0	1	0	0	40	0	0	0	42	29	0	144	
4:35 PM	0	0	0	0	37	0	1	0	0	35	0	0	1	29	24	0	127	
4:40 PM	0	0	0	0	26	0	0	0	1	37	0	0	0	41	28	0	133	
4:45 PM	0	0	0	0	33	0	2	0	1	41	0	0	0	35	28	0	140	
4:50 PM	0	0	1	0	34	0	1	0	0	47	0	0	0	47	21	0	151	
4:55 PM	0	0	0	0	43	0	0	0	1	51	0	0	0	41	26	0	162	1599
5:00 PM	0	0	0	0	38	0	1	0	0	47	0	0	0	40	40	0	166	1655
5:05 PM	0	0	0	0	37	0	0	0	0	42	0	0	1	37	25	0	142	1665
5:10 PM	0	0	0	0	15	0	1	0	2	40	0	0	0	32	28	0	118	1658
5:15 PM	0	0	0	0	31	0	0	0	0	50	0	0	0	36	36	0	153	1693
5:20 PM	0	0	0	0	31	0	1	0	0	37	0	0	0	51	30	0	150	1716
5:25 PM	0	0	0	0	33	0	0	0	0	39	0	0	0	34	32	0	138	1724
5:30 PM	0	0	0	0	32	0	1	0	1	48	0	0	0	40	29	0	151	1731
5:35 PM	0	0	0	0	30	0	0	0	0	40	0	0	0	40	29	0	139	1743
5:40 PM	0	0	0	0	30	0	1	0	0	52	0	0	0	39	30	0	152	1762
5:45 PM	0	0	0	0	30	0	0	0	0	33	0	0	0	39	32	0	134	1756
5:50 PM	0	0	0	0	24	0	0	0	0	44	0	0	0	35	26	0	129	1734
5:55 PM	0	0	0	0	25	0	0	0	0	41	0	0	0	27	26	0	119	1691
Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound					
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Total	
All Vehicles	0	0	4	0	460	0	8	0	4	580	0	0	0	512	348	0	1916	
Heavy Trucks	0	0	4	0	20	0	0	0	4	40	0	0	0	4	12	0	84	
Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Bicycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Railroad	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Stopped Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	

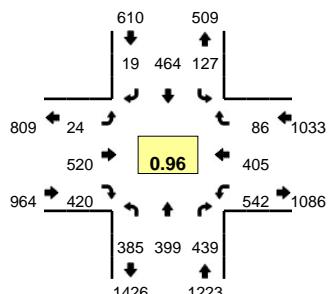
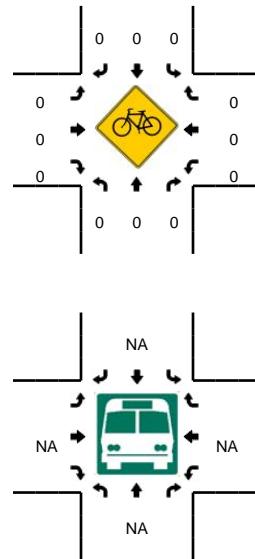
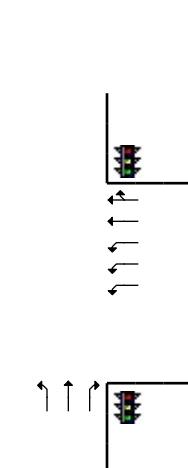
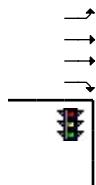
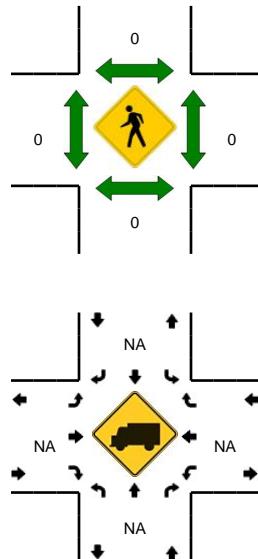
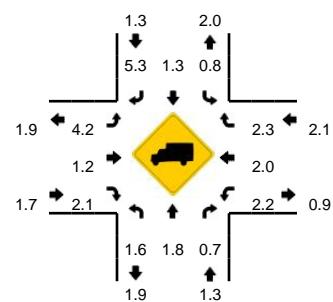
Comments:

Report generated on 1/31/2017 11:25 AM

SOURCE: Quality Counts, LLC (<http://www.qualitycounts.net>) 1-877-580-2212

Type of peak hour being reported: User-Defined

Method for determining peak hour: Total Entering Volume

LOCATION: SW Roy Rogers Rd -- SW Scholls Ferry Rd
CITY/STATE: Sherwood, OR
QC JOB #: 14075210**DATE:** Tue, Dec 06 2016
Peak-Hour: 4:45 PM -- 5:45 PM
Peak 15-Min: 4:50 PM -- 5:05 PM


5-Min Count Period Beginning At	SW Roy Rogers Rd (Northbound)				SW Roy Rogers Rd (Southbound)				SW Scholls Ferry Rd (Eastbound)				SW Scholls Ferry Rd (Westbound)				Total	Hourly Totals
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
4:00 PM	36	44	44	0	9	44	3	0	1	15	27	0	35	22	7	0	287	
4:05 PM	23	27	26	0	4	28	2	0	1	32	27	0	44	49	9	0	272	
4:10 PM	33	25	23	0	4	27	4	0	0	34	21	0	39	30	8	0	248	
4:15 PM	29	35	37	0	10	40	3	0	2	36	21	0	40	27	5	0	285	
4:20 PM	28	36	27	0	6	33	2	0	5	28	33	0	49	31	13	0	291	
4:25 PM	35	22	39	0	14	24	0	0	1	39	33	0	53	40	7	0	307	
4:30 PM	34	43	34	0	10	48	1	0	1	35	30	0	35	27	2	0	300	
4:35 PM	25	28	40	0	11	37	3	0	3	39	32	0	55	26	2	0	301	
4:40 PM	24	33	29	0	4	36	1	0	2	37	42	0	49	44	6	0	307	
4:45 PM	37	30	36	0	13	39	1	0	0	35	38	0	43	34	12	0	318	
4:50 PM	30	48	31	0	12	45	5	0	1	31	42	0	39	25	8	0	317	
4:55 PM	30	37	49	0	9	44	1	0	4	38	42	0	44	35	6	0	339	3572
5:00 PM	30	34	36	0	8	21	2	0	2	56	50	0	48	46	9	0	342	3627
5:05 PM	37	32	33	0	19	36	3	0	2	43	50	0	31	24	7	0	317	3672
5:10 PM	34	32	49	0	18	50	1	0	2	28	27	0	41	31	5	0	318	3742
5:15 PM	30	37	40	0	7	41	0	0	4	37	17	0	60	42	0	0	315	3772
5:20 PM	29	26	38	0	4	33	3	0	0	55	36	0	48	27	7	0	306	3787
5:25 PM	38	22	26	0	16	41	0	0	4	44	29	0	39	38	9	0	306	3786
5:30 PM	33	33	25	0	9	42	1	0	1	50	29	0	42	29	7	0	301	3787
5:35 PM	28	36	45	0	5	50	1	0	4	38	28	0	47	35	6	0	323	3809
5:40 PM	29	32	31	0	7	22	1	0	0	65	32	0	60	39	10	0	328	3830
5:45 PM	34	28	41	0	14	34	1	0	1	47	29	0	41	33	4	0	307	3819
5:50 PM	39	42	38	0	5	38	0	0	2	27	22	0	40	21	3	0	277	3779
5:55 PM	27	38	36	0	11	46	1	0	1	32	31	0	61	27	5	0	316	3756
Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound					
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Total	
All Vehicles	360	476	464	0	116	440	32	0	28	500	536	0	524	424	92	0	3992	
Heavy Trucks	0	12	0	0	4	8	0	0	0	8	28	0	12	12	4	0	88	
Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Bicycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Railroad	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Stopped Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	

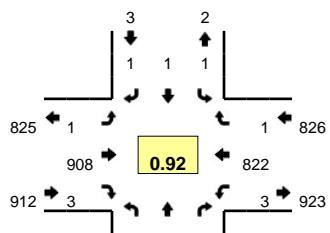
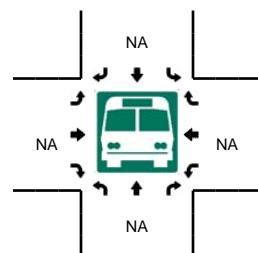
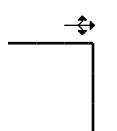
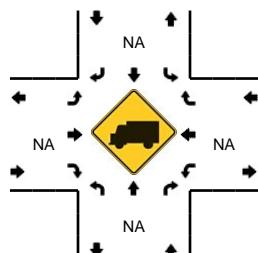
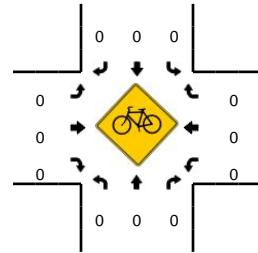
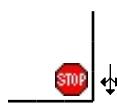
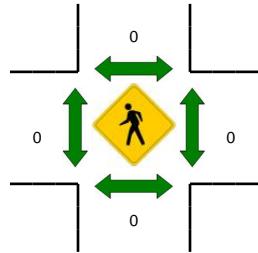
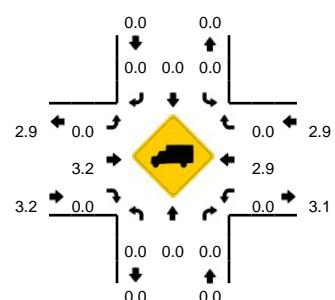
Comments:

Report generated on 1/31/2017 11:25 AM

SOURCE: Quality Counts, LLC (<http://www.qualitycounts.net>) 1-877-580-2212

Type of peak hour being reported: User-Defined

Method for determining peak hour: Total Entering Volume

LOCATION: Strobel Rd/SW Vandermost Rd -- SW Scholls Ferry Rd
CITY/STATE: Beaverton, OR
QC JOB #: 14075216**DATE:** Tue, Dec 06 2016
Peak-Hour: 4:45 PM -- 5:45 PM
Peak 15-Min: 4:50 PM -- 5:05 PM


5-Min Count Period Beginning At	Strobel Rd/SW Vandermost Rd (Northbound)				Strobel Rd/SW Vandermost Rd (Southbound)				SW Scholls Ferry Rd (Eastbound)				SW Scholls Ferry Rd (Westbound)				Total	Hourly Totals
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
4:00 PM	0	0	0	0	0	0	0	0	0	49	0	0	5	64	0	0	118	
4:05 PM	1	0	0	0	0	0	0	0	0	66	0	0	1	68	0	0	136	
4:10 PM	0	0	1	0	0	0	0	0	0	51	0	0	2	65	0	0	119	
4:15 PM	0	0	1	0	0	0	0	0	0	54	0	0	1	62	0	0	118	
4:20 PM	0	0	2	0	0	0	0	0	0	65	2	0	0	60	0	0	129	
4:25 PM	0	0	2	0	0	0	0	0	0	66	0	0	1	68	0	0	137	
4:30 PM	0	0	1	0	0	0	0	0	0	74	1	0	0	66	0	0	142	
4:35 PM	0	0	2	0	0	0	0	0	0	62	0	0	0	64	0	0	128	
4:40 PM	0	0	3	0	0	0	0	0	0	70	0	0	0	62	0	0	135	
4:45 PM	0	0	0	0	0	0	0	0	1	68	0	0	0	63	0	0	132	
4:50 PM	0	0	0	0	0	1	1	0	0	85	0	0	0	65	1	0	153	
4:55 PM	1	0	3	0	0	0	0	0	0	89	0	0	1	72	0	0	166	1613
5:00 PM	1	0	1	0	0	0	0	0	0	84	0	0	1	72	0	0	159	1654
5:05 PM	0	0	3	0	0	0	0	0	0	79	0	0	0	68	0	0	150	1668
5:10 PM	0	0	0	0	0	0	0	0	0	60	0	0	0	65	0	0	125	1674
5:15 PM	0	0	1	0	1	0	0	0	0	79	1	0	0	74	0	0	156	1712
5:20 PM	0	0	4	0	0	0	0	0	0	62	1	0	0	71	0	0	138	1721
5:25 PM	0	0	2	0	0	0	0	0	0	73	1	0	1	70	0	0	147	1731
5:30 PM	0	0	0	0	0	0	0	0	0	80	0	0	0	66	0	0	146	1735
5:35 PM	0	0	0	0	0	0	0	0	0	74	0	0	0	71	0	0	145	1752
5:40 PM	0	0	0	0	0	0	0	0	0	75	0	0	0	65	0	0	140	1757
5:45 PM	0	0	0	0	0	0	0	0	0	65	0	0	0	74	0	0	139	1764
5:50 PM	0	0	2	0	0	0	0	0	0	66	0	0	0	60	0	0	128	1739
5:55 PM	0	0	0	0	0	0	0	0	0	68	0	0	0	58	0	0	126	1699
Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound				Total	
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
All Vehicles	8	0	16	0	0	4	4	0	0	1032	0	0	8	836	4	0	1912	
Heavy Trucks	0	0	0	0	0	0	0	0	0	64	0	0	0	16	0	0	80	
Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Bicycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Railroad	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Stopped Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	

Comments:

Report generated on 1/31/2017 11:25 AM

SOURCE: Quality Counts, LLC (<http://www.qualitycounts.net>) 1-877-580-2212

Appendix B Year 2016 Existing Traffic Conditions

Queues

1: Parking Lot/SW Tile Flat Rd & SW Scholls Ferry Rd

01/19/2017



Lane Group	EBL	EBT	WBT	SBT
Lane Group Flow (vph)	4	555	806	320
v/c Ratio	0.03	0.52	0.79	0.68
Control Delay	46.8	10.3	18.4	31.0
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	46.8	10.3	18.4	31.0
Queue Length 50th (ft)	2	139	245	102
Queue Length 95th (ft)	14	233	561	#280
Internal Link Dist (ft)		230	1105	390
Turn Bay Length (ft)	130			
Base Capacity (vph)	120	1569	1424	618
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.03	0.35	0.57	0.52

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis
1: Parking Lot/SW Tile Flat Rd & SW Scholls Ferry Rd

01/19/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑			↔		↔	↔			↔	
Traffic Volume (vph)	4	555	0	0	394	412	0	0	0	311	0	9
Future Volume (vph)	4	555	0	0	394	412	0	0	0	311	0	9
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	12	12	12	12	12	12	11	11	11	12	12	12
Grade (%)		4%			2%				0%		0%	
Total Lost time (s)	4.0	5.5			5.5					4.5		
Lane Util. Factor	1.00	1.00			1.00					1.00		
Fr _t	1.00	1.00			0.93					1.00		
Flt Protected	0.95	1.00			1.00					0.95		
Satd. Flow (prot)	1769	1708			1646					1691		
Flt Permitted	0.95	1.00			1.00					0.95		
Satd. Flow (perm)	1769	1708			1646					1691		
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	4	555	0	0	394	412	0	0	0	311	0	9
RTOR Reduction (vph)	0	0	0	0	24	0	0	0	0	0	64	0
Lane Group Flow (vph)	4	555	0	0	782	0	0	0	0	0	256	0
Heavy Vehicles (%)	0%	9%	0%	0%	11%	2%	0%	0%	0%	6%	0%	33%
Turn Type	Prot	NA			NA					Split	NA	
Protected Phases	5	2			6		8	8		4	4	
Permitted Phases					6							
Actuated Green, G (s)	0.7	53.0			48.3					19.2		
Effective Green, g (s)	0.7	53.0			48.3					19.2		
Actuated g/C Ratio	0.01	0.64			0.59					0.23		
Clearance Time (s)	4.0	5.5			5.5					4.5		
Vehicle Extension (s)	1.5	3.5			3.5					1.5		
Lane Grp Cap (vph)	15	1101			967					394		
v/s Ratio Prot	0.00	c0.32			c0.48					c0.15		
v/s Ratio Perm												
v/c Ratio	0.27	0.50			0.81					0.65		
Uniform Delay, d1	40.5	7.7			13.3					28.5		
Progression Factor	1.00	1.00			1.00					1.00		
Incremental Delay, d2	3.5	0.4			5.2					2.8		
Delay (s)	43.9	8.1			18.5					31.2		
Level of Service	D	A			B					C		
Approach Delay (s)		8.4			18.5		0.0			31.2		
Approach LOS		A			B		A			C		
Intersection Summary												
HCM 2000 Control Delay		17.6			HCM 2000 Level of Service					B		
HCM 2000 Volume to Capacity ratio		0.82										
Actuated Cycle Length (s)		82.2			Sum of lost time (s)					18.0		
Intersection Capacity Utilization		72.1%			ICU Level of Service					C		
Analysis Period (min)		15										
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis
 2: SW Vandermost Rd/SW Strobel Rd & SW Scholls Ferry Rd

01/19/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	855	0	5	813	0	0	0	3	0	0	0
Future Volume (Veh/h)	0	855	0	5	813	0	0	0	3	0	0	0
Sign Control		Free			Free			Stop			Stop	
Grade		-4%			0%			-7%			-4%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	0	855	0	5	813	0	0	0	3	0	0	0
Pedestrians		1										
Lane Width (ft)		12.0										
Walking Speed (ft/s)		3.5										
Percent Blockage		0										
Right turn flare (veh)												
Median type		None			None							
Median storage veh												
Upstream signal (ft)		1185										
pX, platoon unblocked					0.86			0.86	0.86	0.86	0.86	0.86
vC, conflicting volume	813				855			1679	1678	855	1681	1678
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	813				748			1709	1708	748	1711	1708
tC, single (s)	4.1				4.1			7.1	6.5	6.5	7.1	6.5
tC, 2 stage (s)												
tF (s)	2.2				2.2			3.5	4.0	3.6	3.5	4.0
p0 queue free %	100				99			100	100	99	100	100
cM capacity (veh/h)	823				746			62	79	314	61	79
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	855	818	3	0								
Volume Left	0	5	0	0								
Volume Right	0	0	3	0								
cSH	823	746	314	1700								
Volume to Capacity	0.00	0.01	0.01	0.00								
Queue Length 95th (ft)	0	1	1	0								
Control Delay (s)	0.0	0.2	16.6	0.0								
Lane LOS		A	C	A								
Approach Delay (s)	0.0	0.2	16.6	0.0								
Approach LOS		C	A									
Intersection Summary												
Average Delay			0.1									
Intersection Capacity Utilization		57.1%			ICU Level of Service				B			
Analysis Period (min)			15									

Queues

3: SW Roy Rogers Rd/SW 175th Ave & SW Scholls Ferry Rd

01/19/2017

Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	16	429	403	354	514	364	423	476	73	405
v/c Ratio	0.17	0.69	0.53	0.72	0.44	0.83	0.62	0.48	0.50	0.68
Control Delay	55.2	43.4	14.1	49.3	26.4	49.9	29.6	8.8	58.4	43.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	55.2	43.4	14.1	49.3	26.4	49.9	29.6	8.8	58.4	43.7
Queue Length 50th (ft)	9	122	106	101	107	195	200	84	41	119
Queue Length 95th (ft)	37	221	221	193	224	379	375	204	106	217
Internal Link Dist (ft)		686			809		447			409
Turn Bay Length (ft)	170		330	330		300		300	250	
Base Capacity (vph)	107	1010	979	691	1489	695	989	1062	236	934
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.15	0.42	0.41	0.51	0.35	0.52	0.43	0.45	0.31	0.43
Intersection Summary										

HCM Signalized Intersection Capacity Analysis
 3: SW Roy Rogers Rd/SW 175th Ave & SW Scholls Ferry Rd

01/19/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑	↑	↑↑	↑↑		↑	↑	↑	↑	↑↑	
Traffic Volume (vph)	16	429	403	354	427	87	364	423	476	73	391	14
Future Volume (vph)	16	429	403	354	427	87	364	423	476	73	391	14
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)	0%			0%				3%			-2%	
Total Lost time (s)	4.0	5.5	4.0	4.0	5.5		4.0	5.5	4.0	4.0	5.5	
Lane Util. Factor	1.00	0.95	1.00	0.97	0.95		1.00	1.00	1.00	1.00	*0.92	
Frbp, ped/bikes	1.00	1.00	0.99	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Fr _t	1.00	1.00	0.85	1.00	0.97		1.00	1.00	0.85	1.00	0.99	
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1612	3374	1501	3335	3356		1631	1766	1515	1657	3373	
Flt Permitted	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (perm)	1612	3374	1501	3335	3356		1631	1766	1515	1657	3373	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	16	429	403	354	427	87	364	423	476	73	391	14
RTOR Reduction (vph)	0	0	48	0	13	0	0	0	89	0	2	0
Lane Group Flow (vph)	16	429	355	354	501	0	364	423	387	73	403	0
Confl. Peds. (#/hr)							1				1	
Confl. Bikes (#/hr)				2								
Heavy Vehicles (%)	12%	7%	7%	5%	4%	9%	9%	6%	5%	10%	4%	7%
Turn Type	Prot	NA	pm+ov	Prot	NA		Prot	NA	pm+ov	Prot	NA	
Protected Phases	5	2	3	1	6		3	8	1	7	4	
Permitted Phases			2					8				
Actuated Green, G (s)	1.8	20.1	45.0	13.6	31.9		24.9	35.8	49.4	6.6	17.5	
Effective Green, g (s)	1.8	20.1	45.0	13.6	31.9		24.9	35.8	49.4	6.6	17.5	
Actuated g/C Ratio	0.02	0.21	0.47	0.14	0.34		0.26	0.38	0.52	0.07	0.18	
Clearance Time (s)	4.0	5.5	4.0	4.0	5.5		4.0	5.5	4.0	4.0	5.5	
Vehicle Extension (s)	1.0	2.0	1.0	1.0	2.0		1.0	2.0	1.0	1.0	2.0	
Lane Grp Cap (vph)	30	713	710	476	1125		427	664	786	114	620	
v/s Ratio Prot	0.01	c0.13	0.13	c0.11	0.15		c0.22	c0.24	0.07	0.04	0.12	
v/s Ratio Perm			0.11					0.19				
v/c Ratio	0.53	0.60	0.50	0.74	0.45		0.85	0.64	0.49	0.64	0.65	
Uniform Delay, d1	46.2	33.9	17.3	39.1	24.7		33.4	24.3	14.8	43.1	36.0	
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	8.8	1.0	0.2	5.4	0.1		14.6	1.5	0.2	8.8	1.9	
Delay (s)	55.1	34.9	17.5	44.5	24.8		47.9	25.8	14.9	51.9	37.8	
Level of Service	E	C	B	D	C		D	C	B	D	D	
Approach Delay (s)		27.0			32.8			28.1		40.0		
Approach LOS		C			C			C		D		
Intersection Summary												
HCM 2000 Control Delay		30.7				HCM 2000 Level of Service			C			
HCM 2000 Volume to Capacity ratio		0.74										
Actuated Cycle Length (s)		95.1				Sum of lost time (s)			19.0			
Intersection Capacity Utilization		69.5%				ICU Level of Service			C			
Analysis Period (min)		15										
c Critical Lane Group												

Queues

1: Parking Lot/SW Tile Flat Rd & SW Scholls Ferry Rd

01/18/2017



Lane Group	EBL	EBT	WBT	SBT
Lane Group Flow (vph)	5	638	927	367
v/c Ratio	0.05	0.57	0.86	0.80
Control Delay	51.5	10.3	21.7	43.7
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	51.5	10.3	21.7	43.7
Queue Length 50th (ft)	3	177	336	159
Queue Length 95th (ft)	17	234	632	#408
Internal Link Dist (ft)		230	1105	390
Turn Bay Length (ft)	130			
Base Capacity (vph)	194	1613	1424	456
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.03	0.40	0.65	0.80

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis
1: Parking Lot/SW Tile Flat Rd & SW Scholls Ferry Rd

01/18/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑			↔		↔	↔			↔	
Traffic Volume (vph)	4	555	0	0	394	412	0	0	0	311	0	9
Future Volume (vph)	4	555	0	0	394	412	0	0	0	311	0	9
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	12	12	12	12	12	12	11	11	11	12	12	12
Grade (%)		4%			2%				0%		0%	
Total Lost time (s)	4.0	5.5			5.5					4.5		
Lane Util. Factor	1.00	1.00			1.00					1.00		
Fr _t	1.00	1.00			0.93					1.00		
Flt Protected	0.95	1.00			1.00					0.95		
Satd. Flow (prot)	1769	1708			1646					1691		
Flt Permitted	0.95	1.00			1.00					0.95		
Satd. Flow (perm)	1769	1708			1646					1691		
Peak-hour factor, PHF	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87
Adj. Flow (vph)	5	638	0	0	453	474	0	0	0	357	0	10
RTOR Reduction (vph)	0	0	0	0	25	0	0	0	0	0	65	0
Lane Group Flow (vph)	5	638	0	0	902	0	0	0	0	0	302	0
Heavy Vehicles (%)	0%	9%	0%	0%	11%	2%	0%	0%	0%	6%	0%	33%
Turn Type	Prot	NA			NA					Split	NA	
Protected Phases	5	2			6		8	8		4	4	
Permitted Phases					6							
Actuated Green, G (s)	0.8	64.9			60.1					21.6		
Effective Green, g (s)	0.8	64.9			60.1					21.6		
Actuated g/C Ratio	0.01	0.67			0.62					0.22		
Clearance Time (s)	4.0	5.5			5.5					4.5		
Vehicle Extension (s)	1.5	3.5			3.5					1.5		
Lane Grp Cap (vph)	14	1148			1025					378		
v/s Ratio Prot	0.00	c0.37			c0.55					c0.18		
v/s Ratio Perm												
v/c Ratio	0.36	0.56			0.88					0.80		
Uniform Delay, d1	47.6	8.3			15.2					35.4		
Progression Factor	1.00	1.00			1.00					1.00		
Incremental Delay, d2	5.6	0.6			9.1					10.5		
Delay (s)	53.2	8.9			24.3					45.9		
Level of Service	D	A			C					D		
Approach Delay (s)	9.3				24.3		0.0			45.9		
Approach LOS		A			C		A			D		
Intersection Summary												
HCM 2000 Control Delay	23.4				HCM 2000 Level of Service					C		
HCM 2000 Volume to Capacity ratio	0.91											
Actuated Cycle Length (s)	96.5				Sum of lost time (s)					18.0		
Intersection Capacity Utilization	72.1%				ICU Level of Service					C		
Analysis Period (min)		15										
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis
 2: SW Vandermost Rd/SW Strobel Rd & SW Scholls Ferry Rd

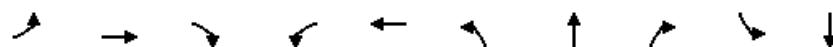
01/18/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	855	0	5	813	0	0	0	3	0	0	0
Future Volume (Veh/h)	0	855	0	5	813	0	0	0	3	0	0	0
Sign Control		Free			Free			Stop			Stop	
Grade		-4%			0%			-7%			-4%	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Hourly flow rate (vph)	0	972	0	6	924	0	0	0	3	0	0	0
Pedestrians		1										
Lane Width (ft)		12.0										
Walking Speed (ft/s)		3.5										
Percent Blockage		0										
Right turn flare (veh)												
Median type		None			None							
Median storage veh												
Upstream signal (ft)		1185										
pX, platoon unblocked					0.81			0.81	0.81	0.81	0.81	0.81
vC, conflicting volume	924				972			1909	1908	972	1911	1908
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	924				849			2005	2003	849	2007	2003
tC, single (s)	4.1				4.1			7.1	6.5	6.5	7.1	6.5
tC, 2 stage (s)												
tF (s)	2.2				2.2			3.5	4.0	3.6	3.5	4.0
p0 queue free %	100				99			100	100	99	100	100
cM capacity (veh/h)	748				647			36	49	259	36	49
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	972	930	3	0								
Volume Left	0	6	0	0								
Volume Right	0	0	3	0								
cSH	748	647	259	1700								
Volume to Capacity	0.00	0.01	0.01	0.00								
Queue Length 95th (ft)	0	1	1	0								
Control Delay (s)	0.0	0.3	19.1	0.0								
Lane LOS		A	C	A								
Approach Delay (s)	0.0	0.3	19.1	0.0								
Approach LOS		C	A									
Intersection Summary												
Average Delay			0.2									
Intersection Capacity Utilization		57.1%			ICU Level of Service				B			
Analysis Period (min)			15									

Queues

3: SW Roy Rogers Rd/SW 175th Ave & SW Scholls Ferry Rd

01/18/2017



Lane Group	EBL	EBT	EBC	WBL	WBT	NBL	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	18	477	448	393	571	404	470	529	81	450
v/c Ratio	0.20	0.75	0.59	0.75	0.48	0.92	0.70	0.53	0.56	0.73
Control Delay	50.7	44.1	16.3	49.3	25.9	64.5	33.5	9.7	57.4	44.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	50.7	44.1	16.3	49.3	25.9	64.5	33.5	9.7	57.4	44.2
Queue Length 50th (ft)	10	141	133	115	122	233	239	96	47	137
Queue Length 95th (ft)	36	210	249	#212	220	#489	#450	237	100	206
Internal Link Dist (ft)		686			809		447			409
Turn Bay Length (ft)	170		330	330		300		300	250	
Base Capacity (vph)	172	1265	759	536	1451	437	760	1000	266	1087
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.10	0.38	0.59	0.73	0.39	0.92	0.62	0.53	0.30	0.41

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis
 3: SW Roy Rogers Rd/SW 175th Ave & SW Scholls Ferry Rd

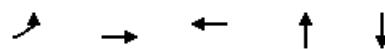
01/18/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑	↑	↑↑	↑↑		↑	↑	↑	↑	↑↑	
Traffic Volume (vph)	16	429	403	354	427	87	364	423	476	73	391	14
Future Volume (vph)	16	429	403	354	427	87	364	423	476	73	391	14
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)	0%			0%				3%			-2%	
Total Lost time (s)	4.0	5.5	4.0	4.0	5.5		4.0	5.5	4.0	4.0	5.5	
Lane Util. Factor	1.00	0.95	1.00	0.97	0.95		1.00	1.00	1.00	1.00	*0.92	
Frbp, ped/bikes	1.00	1.00	0.99	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Fr _t	1.00	1.00	0.85	1.00	0.97		1.00	1.00	0.85	1.00	0.99	
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1612	3374	1501	3335	3355		1631	1766	1515	1657	3372	
Flt Permitted	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (perm)	1612	3374	1501	3335	3355		1631	1766	1515	1657	3372	
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	18	477	448	393	474	97	404	470	529	81	434	16
RTOR Reduction (vph)	0	0	49	0	13	0	0	0	105	0	2	0
Lane Group Flow (vph)	18	477	399	393	558	0	404	470	424	81	448	0
Confl. Peds. (#/hr)							1				1	
Confl. Bikes (#/hr)				2								
Heavy Vehicles (%)	12%	7%	7%	5%	4%	9%	9%	6%	5%	10%	4%	7%
Turn Type	Prot	NA	pm+ov	Prot	NA		Prot	NA	pm+ov	Prot	NA	
Protected Phases	5	2	3	1	6		3	8	1	7	4	
Permitted Phases			2					8				
Actuated Green, G (s)	2.2	20.4	45.6	14.8	33.0		25.2	36.0	50.8	7.1	17.9	
Effective Green, g (s)	2.2	20.4	45.6	14.8	33.0		25.2	36.0	50.8	7.1	17.9	
Actuated g/C Ratio	0.02	0.21	0.47	0.15	0.34		0.26	0.37	0.52	0.07	0.18	
Clearance Time (s)	4.0	5.5	4.0	4.0	5.5		4.0	5.5	4.0	4.0	5.5	
Vehicle Extension (s)	1.0	2.0	1.0	1.0	2.0		1.0	2.0	1.0	1.0	2.0	
Lane Grp Cap (vph)	36	707	703	507	1137		422	653	790	120	620	
v/s Ratio Prot	0.01	c0.14	0.15	c0.12	0.17		c0.25	c0.27	0.08	0.05	0.13	
v/s Ratio Perm			0.12					0.20				
v/c Ratio	0.50	0.67	0.57	0.78	0.49		0.96	0.72	0.54	0.68	0.72	
Uniform Delay, d1	47.0	35.4	18.7	39.7	25.5		35.5	26.3	15.4	44.0	37.4	
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	3.9	2.0	0.6	6.7	0.1		32.4	3.2	0.4	11.2	3.5	
Delay (s)	50.9	37.4	19.3	46.3	25.6		67.9	29.5	15.8	55.1	40.9	
Level of Service	D	D	B	D	C		E	C	B	E	D	
Approach Delay (s)			29.1		34.0			35.4			43.1	
Approach LOS		C			C			D			D	
Intersection Summary												
HCM 2000 Control Delay			34.6			HCM 2000 Level of Service			C			
HCM 2000 Volume to Capacity ratio			0.82									
Actuated Cycle Length (s)			97.3			Sum of lost time (s)			19.0			
Intersection Capacity Utilization			69.5%			ICU Level of Service			C			
Analysis Period (min)			15									
c Critical Lane Group												

Queues

1: Parking Lot/SW Tile Flat Rd & SW Scholls Ferry Rd

01/19/2017



Lane Group	EBL	EBT	WBT	NBT	SBT
Lane Group Flow (vph)	5	534	827	1	395
v/c Ratio	0.06	0.50	0.82	0.00	0.71
Control Delay	51.4	12.7	23.8	0.0	33.6
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	51.4	12.7	23.8	0.0	33.6
Queue Length 50th (ft)	3	162	319	0	153
Queue Length 95th (ft)	18	284	712	0	#423
Internal Link Dist (ft)		230	1105	30	390
Turn Bay Length (ft)		130			
Base Capacity (vph)	85	1535	1351	373	603
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.06	0.35	0.61	0.00	0.66

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis
1: Parking Lot/SW Tile Flat Rd & SW Scholls Ferry Rd

01/19/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑			↔			↔			↔	
Traffic Volume (vph)	5	534	0	1	472	354	0	0	1	387	0	8
Future Volume (vph)	5	534	0	1	472	354	0	0	1	387	0	8
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	12	12	12	12	12	12	11	11	11	12	12	12
Grade (%)		4%			2%			0%			0%	
Total Lost time (s)	4.0	5.5			5.5			4.0			4.5	
Lane Util. Factor	1.00	1.00			1.00			1.00			1.00	
Fr _t	1.00	1.00			0.94			0.86			1.00	
Flt Protected	0.95	1.00			1.00			1.00			0.95	
Satd. Flow (prot)	1474	1790			1718			794			1772	
Flt Permitted	0.95	1.00			1.00			1.00			0.95	
Satd. Flow (perm)	1474	1790			1718			794			1772	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	5	534	0	1	472	354	0	0	1	387	0	8
RTOR Reduction (vph)	0	0	0	0	18	0	0	1	0	0	62	0
Lane Group Flow (vph)	5	534	0	0	809	0	0	0	0	0	333	0
Heavy Vehicles (%)	20%	4%	0%	0%	4%	2%	0%	0%	100%	2%	0%	0%
Turn Type	Prot	NA		Perm	NA			NA		Split	NA	
Protected Phases	5	2			6			8	8	4	4	
Permitted Phases					6							
Actuated Green, G (s)	0.7	56.5			51.8			0.6			25.1	
Effective Green, g (s)	0.7	56.5			51.8			0.6			25.1	
Actuated g/C Ratio	0.01	0.59			0.54			0.01			0.26	
Clearance Time (s)	4.0	5.5			5.5			4.0			4.5	
Vehicle Extension (s)	1.5	3.5			3.5			1.5			1.5	
Lane Grp Cap (vph)	10	1051			925			4			462	
v/s Ratio Prot	0.00	c0.30						c0.00			c0.19	
v/s Ratio Perm					0.47							
v/c Ratio	0.50	0.51			0.87			0.00			0.72	
Uniform Delay, d1	47.6	11.7			19.4			47.5			32.4	
Progression Factor	1.00	1.00			1.00			1.00			1.00	
Incremental Delay, d2	13.6	0.5			9.4			0.1			4.7	
Delay (s)	61.2	12.1			28.8			47.6			37.0	
Level of Service	E	B			C			D			D	
Approach Delay (s)		12.6			28.8			47.6			37.0	
Approach LOS		B			C			D			D	
Intersection Summary												
HCM 2000 Control Delay		25.7			HCM 2000 Level of Service			C				
HCM 2000 Volume to Capacity ratio		0.82										
Actuated Cycle Length (s)		96.2			Sum of lost time (s)			18.0				
Intersection Capacity Utilization		84.2%			ICU Level of Service			E				
Analysis Period (min)		15										
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis
 2: SW Vandermost Rd/SW Strobel Rd & SW Scholls Ferry Rd

01/19/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔			↔	
Traffic Volume (veh/h)	1	908	3	3	822	1	2	0	14	1	1	1
Future Volume (Veh/h)	1	908	3	3	822	1	2	0	14	1	1	1
Sign Control		Free			Free			Stop			Stop	
Grade		-4%			0%			-7%			-4%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	1	908	3	3	822	1	2	0	14	1	1	1
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage veh												
Upstream signal (ft)		1185										
pX, platoon unblocked					0.83			0.83	0.83	0.83	0.83	0.83
vC, conflicting volume	823				911			1742	1740	910	1754	1742
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	823				790			1791	1790	788	1806	1791
tC, single (s)	4.1				4.1			7.1	6.5	6.2	7.1	6.5
tC, 2 stage (s)												
tF (s)	2.2				2.2			3.5	4.0	3.3	3.5	4.0
p0 queue free %	100				100			96	100	96	98	99
cM capacity (veh/h)	816				696			52	68	328	49	68
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	912	826	16	3								
Volume Left	1	3	2	1								
Volume Right	3	1	14	1								
cSH	816	696	197	80								
Volume to Capacity	0.00	0.00	0.08	0.04								
Queue Length 95th (ft)	0	0	7	3								
Control Delay (s)	0.0	0.1	24.9	52.0								
Lane LOS	A	A	C	F								
Approach Delay (s)	0.0	0.1	24.9	52.0								
Approach LOS			C	F								
Intersection Summary												
Average Delay			0.4									
Intersection Capacity Utilization		58.7%			ICU Level of Service				B			
Analysis Period (min)			15									

Queues

3: SW Roy Rogers Rd/SW 175th Ave & SW Scholls Ferry Rd

01/19/2017

Lane Group	EBL	EBT	EBC	WBL	WBT	NBL	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	24	520	420	542	491	385	399	439	127	483
v/c Ratio	0.26	0.77	0.50	0.83	0.38	0.87	0.66	0.46	0.67	0.75
Control Delay	62.1	50.6	16.3	54.9	26.4	59.6	37.8	10.7	66.5	50.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	62.1	50.6	16.3	54.9	26.4	59.6	37.8	10.7	66.5	50.4
Queue Length 50th (ft)	17	186	146	191	133	258	238	107	89	172
Queue Length 95th (ft)	48	271	255	#300	201	#452	387	211	163	251
Internal Link Dist (ft)		686			809		447			409
Turn Bay Length (ft)	170		330	330		300		300	250	
Base Capacity (vph)	103	874	943	798	1452	571	748	1023	288	857
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.23	0.59	0.45	0.68	0.34	0.67	0.53	0.43	0.44	0.56

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis
 3: SW Roy Rogers Rd/SW 175th Ave & SW Scholls Ferry Rd

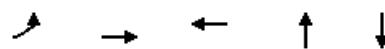
01/19/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑	↑	↑↑	↑↑		↑	↑	↑	↑	↑↑	
Traffic Volume (vph)	24	520	420	542	405	86	385	399	439	127	464	19
Future Volume (vph)	24	520	420	542	405	86	385	399	439	127	464	19
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)		0%			0%			3%			-2%	
Total Lost time (s)	4.0	5.5	4.0	4.0	5.5		4.0	5.5	4.0	4.0	5.5	
Lane Util. Factor	1.00	0.95	1.00	0.97	0.95		1.00	1.00	1.00	1.00	0.95	
Fr _t	1.00	1.00	0.85	1.00	0.97		1.00	1.00	0.85	1.00	0.99	
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1736	3574	1583	3433	3446		1743	1835	1575	1805	3583	
Flt Permitted	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (perm)	1736	3574	1583	3433	3446		1743	1835	1575	1805	3583	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	24	520	420	542	405	86	385	399	439	127	464	19
RTOR Reduction (vph)	0	0	50	0	14	0	0	0	68	0	2	0
Lane Group Flow (vph)	24	520	370	542	477	0	385	399	371	127	481	0
Heavy Vehicles (%)	4%	1%	2%	2%	2%	2%	2%	2%	1%	1%	1%	5%
Turn Type	Prot	NA	pm+ov	Prot	NA		Prot	NA	pm+ov	Prot	NA	
Protected Phases	5	2	3	1	6		3	8	1	7	4	
Permitted Phases			2						8			
Actuated Green, G (s)	3.0	21.8	48.4	19.8	38.6		26.6	34.4	54.2	11.0	18.8	
Effective Green, g (s)	3.0	21.8	48.4	19.8	38.6		26.6	34.4	54.2	11.0	18.8	
Actuated g/C Ratio	0.03	0.21	0.46	0.19	0.36		0.25	0.32	0.51	0.10	0.18	
Clearance Time (s)	4.0	5.5	4.0	4.0	5.5		4.0	5.5	4.0	4.0	5.5	
Vehicle Extension (s)	1.0	2.0	1.0	1.0	2.0		1.0	2.0	1.0	1.0	2.0	
Lane Grp Cap (vph)	49	735	722	641	1254		437	595	805	187	635	
v/s Ratio Prot	0.01	c0.15	0.13	c0.16	0.14		c0.22	c0.22	0.09	0.07	0.13	
v/s Ratio Perm			0.11						0.15			
v/c Ratio	0.49	0.71	0.51	0.85	0.38		0.88	0.67	0.46	0.68	0.76	
Uniform Delay, d1	50.7	39.1	20.4	41.6	24.9		38.2	30.9	16.6	45.8	41.4	
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	2.8	2.6	0.3	9.6	0.1		17.9	2.3	0.2	7.5	4.6	
Delay (s)	53.5	41.7	20.7	51.2	24.9		56.1	33.2	16.7	53.3	46.0	
Level of Service	D	D	C	D	C		E	C	B	D	D	
Approach Delay (s)		32.8			38.7			34.5			47.5	
Approach LOS		C			D			C			D	
Intersection Summary												
HCM 2000 Control Delay		37.3										
HCM 2000 Volume to Capacity ratio		0.80										
Actuated Cycle Length (s)		106.0										
Intersection Capacity Utilization		80.4%										
Analysis Period (min)		15										
c Critical Lane Group												

Queues

1: Parking Lot/SW Tile Flat Rd & SW Scholls Ferry Rd

01/18/2017



Lane Group	EBL	EBT	WBT	NBT	SBT
Lane Group Flow (vph)	5	580	899	1	430
v/c Ratio	0.05	0.53	0.87	0.00	0.83
Control Delay	50.4	11.2	24.5	0.0	42.4
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	50.4	11.2	24.5	0.0	42.4
Queue Length 50th (ft)	2	149	307	0	160
Queue Length 95th (ft)	18	270	749	0	#582
Internal Link Dist (ft)		230	1105	30	390
Turn Bay Length (ft)		130			
Base Capacity (vph)	181	1693	1562	349	519
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.03	0.34	0.58	0.00	0.83

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis
1: Parking Lot/SW Tile Flat Rd & SW Scholls Ferry Rd

01/18/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑			↔			↔			↔	
Traffic Volume (vph)	5	534	0	1	472	354	0	0	1	387	0	8
Future Volume (vph)	5	534	0	1	472	354	0	0	1	387	0	8
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	12	12	12	12	12	12	11	11	11	12	12	12
Grade (%)		4%			2%			0%			0%	
Total Lost time (s)	4.0	5.5			5.5			4.0			4.5	
Lane Util. Factor	1.00	1.00			1.00			1.00			1.00	
Fr _t	1.00	1.00			0.94			0.86			1.00	
Flt Protected	0.95	1.00			1.00			1.00			0.95	
Satd. Flow (prot)	1474	1790			1718			794			1772	
Flt Permitted	0.95	1.00			1.00			1.00			0.95	
Satd. Flow (perm)	1474	1790			1718			794			1772	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	5	580	0	1	513	385	0	0	1	421	0	9
RTOR Reduction (vph)	0	0	0	0	21	0	0	1	0	0	64	0
Lane Group Flow (vph)	5	580	0	0	878	0	0	0	0	0	366	0
Heavy Vehicles (%)	20%	4%	0%	0%	4%	2%	0%	0%	100%	2%	0%	0%
Turn Type	Prot	NA		Perm	NA			NA		Split	NA	
Protected Phases	5	2			6			8	8	4	4	
Permitted Phases					6							
Actuated Green, G (s)	0.8	55.7			50.9			0.5			22.3	
Effective Green, g (s)	0.8	55.7			50.9			0.5			22.3	
Actuated g/C Ratio	0.01	0.60			0.55			0.01			0.24	
Clearance Time (s)	4.0	5.5			5.5			4.0			4.5	
Vehicle Extension (s)	1.5	3.5			3.5			1.5			1.5	
Lane Grp Cap (vph)	12	1077			945			4			427	
v/s Ratio Prot	0.00	c0.32						c0.00			c0.21	
v/s Ratio Perm					0.51							
v/c Ratio	0.42	0.54			0.93			0.00			0.86	
Uniform Delay, d1	45.6	10.8			19.1			45.8			33.6	
Progression Factor	1.00	1.00			1.00			1.00			1.00	
Incremental Delay, d2	8.3	0.6			15.0			0.0			15.0	
Delay (s)	53.9	11.4			34.2			45.8			48.6	
Level of Service	D	B			C			D			D	
Approach Delay (s)		11.8			34.2			45.8			48.6	
Approach LOS		B			C			D			D	
Intersection Summary												
HCM 2000 Control Delay		30.6			HCM 2000 Level of Service			C				
HCM 2000 Volume to Capacity ratio		0.91										
Actuated Cycle Length (s)		92.5			Sum of lost time (s)			18.0				
Intersection Capacity Utilization		84.2%			ICU Level of Service			E				
Analysis Period (min)		15										
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis
 2: SW Vandermost Rd/SW Strobel Rd & SW Scholls Ferry Rd

01/18/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔			↔	
Traffic Volume (veh/h)	1	908		3	3	822	1	2	0	14	1	1
Future Volume (Veh/h)	1	908		3	3	822	1	2	0	14	1	1
Sign Control		Free				Free			Stop		Stop	
Grade		-4%				0%			-7%		-4%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	1	987		3	3	893	1	2	0	15	1	1
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None				None						
Median storage veh)												
Upstream signal (ft)		1185										
pX, platoon unblocked					0.81			0.81	0.81	0.81	0.81	0.81
vC, conflicting volume	894				990			1892	1890	988	1905	1892
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	894				869			1984	1983	867	2001	1984
tC, single (s)	4.1				4.1			7.1	6.5	6.2	7.1	6.5
tC, 2 stage (s)												
tF (s)	2.2				2.2			3.5	4.0	3.3	3.5	4.0
p0 queue free %	100				100			95	100	95	97	98
cM capacity (veh/h)	767				634			37	50	288	35	50
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	991	897	17	3								
Volume Left	1	3	2	1								
Volume Right	3	1	15	1								
cSH	767	634	160	58								
Volume to Capacity	0.00	0.00	0.11	0.05								
Queue Length 95th (ft)	0	0	9	4								
Control Delay (s)	0.0	0.1	30.2	70.6								
Lane LOS	A	A	D	F								
Approach Delay (s)	0.0	0.1	30.2	70.6								
Approach LOS			D	F								
Intersection Summary												
Average Delay			0.5									
Intersection Capacity Utilization		58.7%			ICU Level of Service				B			
Analysis Period (min)			15									

Queues

3: SW Roy Rogers Rd/SW 175th Ave & SW Scholls Ferry Rd

01/18/2017



Lane Group	EBL	EBT	EBC	WBL	WBT	NBL	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	25	542	438	565	512	401	416	457	132	503
v/c Ratio	0.25	0.76	0.51	1.06	0.41	0.89	0.67	0.49	0.68	0.75
Control Delay	53.1	44.3	14.6	95.7	25.0	59.0	35.3	12.2	60.5	45.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	53.1	44.3	14.6	95.7	25.0	59.0	35.3	12.2	60.5	45.1
Queue Length 50th (ft)	15	165	128	~194	108	234	215	108	78	153
Queue Length 95th (ft)	46	241	242	#364	198	#495	377	240	153	227
Internal Link Dist (ft)		686			809		447			409
Turn Bay Length (ft)	170		330	330		300		300	250	
Base Capacity (vph)	180	1301	863	535	1447	453	763	931	281	1120
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.14	0.42	0.51	1.06	0.35	0.89	0.55	0.49	0.47	0.45

Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis
 3: SW Roy Rogers Rd/SW 175th Ave & SW Scholls Ferry Rd

01/18/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑	↑	↑↑	↑↑		↑	↑	↑	↑	↑↑	
Traffic Volume (vph)	24	520	420	542	405	86	385	399	439	127	464	19
Future Volume (vph)	24	520	420	542	405	86	385	399	439	127	464	19
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)		0%			0%			3%			-2%	
Total Lost time (s)	4.0	5.5	4.0	4.0	5.5		4.0	5.5	4.0	4.0	5.5	
Lane Util. Factor	1.00	0.95	1.00	0.97	0.95		1.00	1.00	1.00	1.00	0.95	
Fr _t	1.00	1.00	0.85	1.00	0.97		1.00	1.00	0.85	1.00	0.99	
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1736	3574	1583	3433	3446		1743	1835	1575	1805	3583	
Flt Permitted	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (perm)	1736	3574	1583	3433	3446		1743	1835	1575	1805	3583	
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	25	542	438	565	422	90	401	416	457	132	483	20
RTOR Reduction (vph)	0	0	48	0	14	0	0	0	71	0	2	0
Lane Group Flow (vph)	25	542	390	565	498	0	401	416	386	132	501	0
Heavy Vehicles (%)	4%	1%	2%	2%	2%	2%	2%	2%	1%	1%	1%	5%
Turn Type	Prot	NA	pm+ov	Prot	NA		Prot	NA	pm+ov	Prot	NA	
Protected Phases	5	2	3	1	6		3	8	1	7	4	
Permitted Phases			2					8				
Actuated Green, G (s)	2.4	22.0	47.2	15.1	34.7		25.2	32.9	48.0	10.4	18.1	
Effective Green, g (s)	2.4	22.0	47.2	15.1	34.7		25.2	32.9	48.0	10.4	18.1	
Actuated g/C Ratio	0.02	0.22	0.47	0.15	0.35		0.25	0.33	0.48	0.10	0.18	
Clearance Time (s)	4.0	5.5	4.0	4.0	5.5		4.0	5.5	4.0	4.0	5.5	
Vehicle Extension (s)	1.0	2.0	1.0	1.0	2.0		1.0	2.0	1.0	1.0	2.0	
Lane Grp Cap (vph)	41	791	751	521	1202		441	607	760	188	652	
v/s Ratio Prot	0.01	c0.15	0.13	c0.16	0.14		c0.23	c0.23	0.08	0.07	0.14	
v/s Ratio Perm			0.11					0.17				
v/c Ratio	0.61	0.69	0.52	1.08	0.41		0.91	0.69	0.51	0.70	0.77	
Uniform Delay, d1	48.0	35.5	18.2	42.2	24.6		36.0	28.8	17.6	43.0	38.7	
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	16.3	2.0	0.3	64.2	0.1		21.9	2.6	0.2	9.3	4.9	
Delay (s)	64.4	37.5	18.4	106.4	24.7		57.8	31.3	17.8	52.3	43.5	
Level of Service	E	D	B	F	C		E	C	B	D	D	
Approach Delay (s)		29.9			67.6			34.8			45.4	
Approach LOS		C			E			C			D	
Intersection Summary												
HCM 2000 Control Delay		44.1										
HCM 2000 Volume to Capacity ratio		0.85										
Actuated Cycle Length (s)		99.4										
Intersection Capacity Utilization		80.4%										
Analysis Period (min)		15										
c Critical Lane Group												

Appendix C ODOT Crash Data

COLLISION TYPE	FATAL CRASHES	NON-FATAL CRASHES	PROPERTY DAMAGE ONLY	TOTAL CRASHES	PEOPLE KILLED	PEOPLE INJURED	TRUCKS	DRY SURF	WET SURF	DAY	DARK	INTER-SECTION	INTER-SECTION	OFF-RELATED ROAD
YEAR: 2013	0	2	3	5	0	2	1	4	1	4	1	5	5	0
TURNING MOVEMENTS	0	2	3	5	0	2	1	4	1	4	1	5	5	0
2013 TOTAL	0	2	3	5	0	2	1	4	1	4	1	5	5	0
YEAR: 2012	0	1	0	1	0	1	0	1	0	1	0	1	1	0
TURNING MOVEMENTS	0	1	0	1	0	1	0	1	0	1	0	1	1	0
2012 TOTAL	0	1	0	1	0	1	0	1	0	1	0	1	1	0
FINAL TOTAL	0	3	3	6	0	3	1	5	1	5	1	6	6	0

Disclaimer: A higher number of crashes may be reported as of 2011 compared to prior years. This does not reflect an increase in annual crashes. The higher numbers result from a change to an internal departmental process that allows the Crash Analysis and Reporting Unit to add previously unavailable, non-fatal crash reports to the annual data file. Please be aware of this change when comparing pre-2011 crash statistics.

WASHINGTON COUNTY
WASHINGON COUNTYTRANSPORTATION DATA SECTION - CRASH ANALYSIS AND REPORTING UNIT
COUNTY ROAD CRASH LISTING

SW Scholls Ferry Road & SW Tile Flat Road

January 1, 2010 through December 31, 2014

SER# INVEST E UNLOCK? D C S L K	R H R LAT/LONG	S P O DATE	MILEPOST DIST FROM INTERSECT	FIRST STREET SECOND STREET INTERSECTION SEQ #	COUNTY ROADS RD CHAR DIRECT LOCIN	INT-TYP (MEDIAN) LEGS (#LANES)	OFF-RD WTHR RNDST COLL TYP	CRASH TYP SURF DRY/W	TRUQTY OWNER TYPE FROM TO	SPCL USE PRVTE PSNGR CAR	MOVE PRVTE E W	A S PRTC INJ TYPE SURVY E X RES	ACTN EVENT LOC	CAUSE
00606 COUNTY No	N N N N 45 25	2/3/2012 Fri 28.97 -122 52	7A 0 18.23	SW SCHOLLS FERRY RD SW TILE FLAT RD	INTER CN 01	3-LEG N STOP SIGN N DAY	N CLR DRY TURN INJ	01 NONE 0 STRGHT PRVTE N E	01 DRVNR INJC 49 M OR-Y	000	000	000	000	
03350 COUNTY No	N N N N 45 25	6/25/2013 Tue 28.97 -122 52	10A 0 18.23	SW SCHOLLS FERRY RD SW TILE FLAT RD	INTER CN 01	3-LEG N STOP SIGN N DAY	N CLR DRY TURN INJ	01 NONE 1 TURN-L PRVTE N E	01 DRVNR NONE 27 M OR-Y	028	028	000	002	
04660 COUNTY No	N N N 45 25	8/22/2013 Thu 28.97 -122 52	7A 0 18.23	SW SCHOLLS FERRY RD SW TILE FLAT RD	INTER CN 01	3-LEG N STOP SIGN N DAY	N CLR DRY TURN INJ	01 NONE 0 TURN-L PRVTE N E	01 DRVNR NONE 33 M OTH-Y	014	000	000	10	
05678 NONE No	N N N 45 25	10/5/2013 Sat 28.97 -122 52	5P 0 18.23	SW SCHOLLS FERRY RD SW TILE FLAT RD	INTER CN 01	3-LEG N STOP SIGN N DAY	N CLR DRY TURN INJ	02 NONE 0 STRGHT PRVTE N E	01 DRVNR NONE 42 M OR-Y	000	000	000	00	
07087 COUNTY No	N N N N 45 25	12/5/2013 Thu 28.97 -122 52	8A 0 18.23	SW SCHOLLS FERRY RD SW TILE FLAT RD	INTER CN 01	3-LEG N STOP SIGN N DAY	N CLR DRY TURN INJ	02 NONE 0 TURN-L PRVTE N E	01 DRVNR NONE 35 F OR-Y	028	028	000	02	

WASHINGTON COUNTY

12/14/2016

WASHINGTON COUNTY

**OREGON DEPARTMENT OF TRANSPORTATION - TRANSPORTATION DEVELOPMENT DIVISION
TRANSPORTATION DATA SECTION - CRASH ANALYSIS AND REPORTING UNIT**

**SW School's Ferry Road & SW Tile Flat Road
COUNTY ROAD CRASH LISTING**

COLLISION TYPE	FATAL CRASHES	NON-FATAL CRASHES	PROPERTY DAMAGE ONLY	TOTAL CRASHES	PEOPLE KILLED	PEOPLE INJURED	TRUCKS	DRY SURF	WET SURF	DAY	DARK	INTER-SECTION	INTER-SECTION	OFF-RELATED ROAD
TURNING MOVEMENTS	0	1	0	1	0	1	0	1	0	1	0	1	1	0
2011 TOTAL	0	1	0	1	0	1	0	1	0	1	0	1	1	0
FINAL TOTAL	0	1	0	1	0	1	0	1	0	1	0	1	1	0

Disclaimer: A higher number of crashes may be reported as of 2011 compared to prior years. This does not reflect an increase in annual crashes. The higher numbers result from a change to an internal departmental process that allows the Crash Analysis and Reporting Unit to add previously unavailable, non-fatal crash reports to the annual data file. Please be aware of this change when comparing pre-2011 crash statistics.

WASHINGTON COUNTY

SW Scholls Ferry Road & SW Strobel Road / SW Vandermost Rd

January 1, 2010 through December 31, 2014

	S	D	R	S	W	COUNTY ROADS	FIRST STREET	MILEPTN	RD CHAR	INT-TYP	SPCL USE
SER#	E	A	I	C	O	DATE	DIST FROM	SECOND STREET	DIRECT	(MEDIAN)	OFF-RD WTHR
INVEST?	E	L	G	H	R	DAY/TIME	INTERSECT	INTERSECTION	LOCIN	INT-REL	CRASH TYP
UNLOCK?	D	C	S	L	K	LAT/LONG	SEQ #	SEQ #	LOCIN	(#LANES)	COLL TYP
03404	N	N	N	6/29/2011	SW SCHOLLS FERRY RD	INTER	CROSS	N	CLD	S-1TURN	01 NONE 0 STRGHT
COUNTY	45	25	32.74	-122	52	2.77	0	SW STROBEL RD	CN	DRY TURN	PRVTE E W
No	0	0	0	0	0	0	0	0	01	DRVR INJC	46 M OR-Y 000 000 000
										PSNGR CAR	OR<25
									02	NONE 0 U-TURN	
									PRVTE E E	PSNGR CAR	
									01	DRVR NONE	35 M OR-Y 008 000 000
										PSNGR CAR	OR<25

COLLISION TYPE		FATAL CRASHES	NON-FATAL CRASHES	PROPERTY DAMAGE ONLY	TOTAL CRASHES	PEOPLE KILLED	PEOPLE INJURED	TRUCKS	DRY SURF	WET SURF	DAY	DARK	INTER-SECTION	INTER-SECTION	OFF-RELATED ROAD
YEAR: 2014															
FIXED / OTHER OBJECT	0	0	1	1	0	0	0	0	0	1	0	1	1	1	0
REAR-END	0	1	3	4	0	1	0	0	3	1	3	1	4	0	0
SIDESWIPE - OVERTAKING	0	0	1	1	0	0	0	0	1	0	0	1	1	0	0
TURNING MOVEMENTS	0	1	0	1	0	1	0	0	0	1	0	1	1	0	0
2014 TOTAL	0	2	5	7	0	2	0	4	3	3	4	7	0	1	
YEAR: 2013															
REAR-END	0	0	1	1	0	0	0	0	1	0	1	0	1	0	0
TURNING MOVEMENTS	0	0	2	2	0	0	0	0	2	1	2	1	2	0	0
2013 TOTAL	0	0	3	3	0	0	0	0	2	1	2	1	3	0	0
YEAR: 2012															
ANGLE	0	2	0	2	0	3	0	1	1	1	0	2	2	0	0
FIXED / OTHER OBJECT	0	1	0	1	0	1	0	0	0	1	0	1	1	0	1
REAR-END	0	3	1	4	0	4	0	4	0	4	0	4	0	0	0
TURNING MOVEMENTS	0	0	1	1	0	0	0	0	1	0	1	0	1	0	0
2012 TOTAL	0	6	2	8	0	8	0	6	2	5	3	8	0	1	
YEAR: 2011															
FIXED / OTHER OBJECT	0	0	1	1	0	0	0	0	1	0	1	1	0	1	0
REAR-END	0	1	3	4	0	1	0	2	1	1	2	2	4	0	0
TURNING MOVEMENTS	0	1	2	2	0	1	0	1	1	1	1	2	0	0	0
2011 TOTAL	0	2	5	7	0	2	0	3	3	3	4	7	0	1	
YEAR: 2010															
REAR-END	0	1	3	4	0	3	0	3	1	4	0	4	0	0	0
2010 TOTAL	0	1	18	29	0	15	0	18	10	17	12	29	0	3	
FINAL TOTAL															

Disclaimer: A higher number of crashes may be reported as of 2011 compared to prior years. This does not reflect an increase in annual crashes. The higher numbers result from a change to an internal departmental process that allows the Crash Analysis and Reporting Unit to add previously unavailable, non-fatal crash reports to the annual data file. Please be aware of this change when comparing pre-2011 crash statistics.

WASHINGTON COUNTY
WASHINTON COUNTY

TRANSPORTATION DATA SECTION - CRASH ANALYSIS AND REPORTING UNIT

COUNTY ROAD CRASH LISTING

SW Scholls Ferry Road & SW Roy Rogers Road / SW 175th Ave

January 1, 2010 through December 31, 2014

SER#	D	P	R	S	W	COUNTY ROADS	MILEPNT	FIRST STREET	SECOND STREET	RD CHAR	INT-TYP	OFF-RD WTHR	CRASH TYP	TRU QTY	MOVE	SPCL USE	A	S		
INVEST	E	A	U	C	O		LOCN?	E L G H R	DAY/TIME	DIRECT	(MEDIAN)	COLL TYP	V#	OWNER TYPE	FROM	PRTC INJ	G E	LICNS PED		
UNLOCK?	D	C	S	L	K	LAT/LONG	LOCN?			LOCN	(#LANES)	DRIVWY	VEH TYPE	TO	D# TYPE	SVRTY	E X RES	ACTN EVENT	CAUSE	
86807	N	N	N	7/7/2013	SW ROY ROGERS RD	INTER	N	CROSS	N	0	0	S-1STOP	01	NONE	0	STRGHT	07	00		
NO RPT	45	25	34.95	-122	51	12.51	0	SW SCHOLLS FERRY RD	0	0.6	0	REAR	PDO	01	DRVNR	NONE	36 F OR-Y	026	00	
No												PSNGR CAR		01	DRVNR	NONE	OR<25		07	
01984	Y	N	N	4/17/2011	SW ROY ROGERS RD	INTER	N	CROSS	N	0.5	0	0	PRVTE	E	02	NONE	0	STOP	011	00
COUNTY					SW SCHOLLS FERRY RD	S	TRF SIGNAL	N	CLR	0.5	0	DRY	PSNGR CAR		01	NONE	0	PRVTE	000	00
No	45	25	34.95	-122	51	12.48						TURN	E		01	DRVNR	NONE	OR<25		01,08
05668	Y	N	N	10/13/2011	SW ROY ROGERS RD	INTER	N	CROSS	N	0.5	0	0	PRVTE	E	02	NONE	0	TURN-L	079,010	00
COUNTY					SW SCHOLLS FERRY RD	S	TRF SIGNAL	N	CLR	0.5	0	DRY	PSNGR CAR		01	NONE	0	PRVTE	000	00
No	45	25	34.95	-122	51	12.48						DL/T	INJ		01	DRVNR	NONE	25 M OR-Y	047,080	01,08
05883	Y	N	N	10/27/2012	SW ROY ROGERS RD	INTER	N	CROSS	N	0.5	0	0	PRVTE	E	02	NONE	0	TURN-L	000	00
COUNTY					SW SCHOLLS FERRY RD	S	TRF SIGNAL	N	CLR	0.5	0	DRY	PSNGR CAR		01	NONE	0	PRVTE	000	00
No	45	25	34.95	-122	51	12.51						DL/T	INJ		01	DRVNR	NONE	17 M OR-Y	047,080,081	017
02472	N	N	N	5/14/2013	SW ROY ROGERS RD	INTER	N	CROSS	N	0.5	0	0	PRVTE	E	02	NONE	0	TURN-L	100,073,124	01
NO RPT	45	25	34.95	-122	51	12.51	0	SW SCHOLLS FERRY RD	0	0.5	0	DRY	PSNGR CAR		01	NONE	0	PRVTE	088,100,073,124	00
No												DL/T	INJ		01	DRVNR	NONE	20 F OR-Y	047,080,081	017
00616	N	N	N	2/9/2010	SW ROY ROGERS RD	INTER	N	CROSS	N	0.6	0	0	PRVTE	E	02	NONE	0	TURN-L	000	00
NONE					SW SCHOLLS FERRY RD	S	TRF SIGNAL	N	CLR	0.6	0	DRY	PSNGR CAR		01	NONE	0	PRVTE	000	00
No	45	25	34.95	-122	51	12.51	1					DL/T	INJ		01	DRVNR	NONE	28 M OR-Y	026	07
												PSNGR CAR		02	NONE	0	STOP	011	00	
															PSNGR CAR		PSNGR CAR	000	00	

WASHINGTON COUNTY
WASHINGON COUNTYTRANSPORTATION DATA SECTION - CRASH ANALYSIS AND REPORTING UNIT
COUNTY ROAD CRASH LISTING

SW Scholls Ferry Road & SW Roy Rogers Road / SW 175th Ave

January 1, 2010 through December 31, 2014

SER#	D	P	R	S	W	COUNTY ROADS	MILEPOST	FIRST STREET	SECOND STREET	RD CHAR	INT-TYP	OFF-RD WTHR	CRASH TYP	TRUQ QTY	MOVE	SPCL USE	A	S	
INVEST	E	A	U	C	O		DIST FROM			DIRECT	(MEDIAN)	COLL TYP	V#	OWNER TYPE	FROM	PRTC INJ	G	LICNS PED	
UNLOCK?	D	C	S	L	K	INTERSECT	SEQ #		LOCN?	LOCN	(#LANES)	DRIVWY	VEH TYPE	TO	D# TYPE SVRTY	E	X RES	ACTN EVENT	CAUSE
03549	N	N	N	N	7/11/2012	SW ROY ROGERS RD	INTER	CROSS	N	INTER	N	CLR	S-1STOP	01	NONE	0	STRGHT		
COUNTY	No	45	25	34.95	-122	51	12.51	SW SCHOLLS FERRY RD	0	W	0	DRVY	REAR	PRVTE	PSNGR CAR	W	E	01 DRVVR NONE	48 F OR-Y
								TRF SIGNAL	N	0.6	0	DAY	INJ	PSNGR CAR	01 DRVVR INJC	37 M OR-Y	000	000	
																	02 NONE	0 STOP	
																	PRVTE	PSNGR CAR	
02614	N	N	5/9/2014	SW ROY ROGERS RD	INTER	CROSS	N	TRF SIGNAL	N	W	0	DRVY	REAR	01	NONE	0	STRGHT		
NO RPT	No	45	25	34.95	-122	51	12.51	SW SCHOLLS FERRY RD	1	0.6	0	DAY	INJ	PRVTE	PSNGR CAR	W	E	01 DRVVR NONE	22 M OR-Y
																	02 NONE	0 STOP	
																	PRVTE	PSNGR CAR	
85917	N	N	6/11/2014	SW ROY ROGERS RD	INTER	CROSS	N	TRF SIGNAL	N	W	0	DRVY	REAR	01	NONE	0	STRGHT		
NONE	No	45	25	34.95	-122	51	12.51	SW SCHOLLS FERRY RD	1	0.6	0	DAY	PDO	PRVTE	PSNGR CAR	W	E	01 DRVVR NONE	35 F OR-Y
																	02 NONE	1 STOP	
																	PRVTE	PSNGR CAR	
04922	N	N	8/26/2014	SW ROY ROGERS RD	INTER	CROSS	N	TRF SIGNAL	N	W	0	DRVY	REAR	01	UNKN	0	STRGHT		
CITY	No	45	25	34.95	-122	51	12.51	SW SCHOLLS FERRY RD	1	0.6	0	DAY	PDO	UNKN	UNKNOWN	W	E	01 DRVVR NONE	50 M OR-Y
																	02 NONE	0 STOP	
																	PRVTE	PSNGR CAR	
06449	N	N	N	11/16/2011	SW ROY ROGERS RD	INTER	CROSS	N	L-GRN-SIG	N	0.3	0	DRVY	TURN-R	01	NONE	0	TURN-R	
COUNTY	No	45	25	34.95	-122	51	12.49	SW SCHOLLS FERRY RD	0	0	0	DAY	PDO	PRVTE	PSNGR CAR	W	S	01 DRVVR NONE	60 F OR-Y
																	02 NONE	0 TURN-L	
																	PRVTE	PSNGR CAR	

ACTION CODE TRANSLATION LIST

ACTION CODE	SHORT DESCRIPTION	LONG DESCRIPTION
000	NONE	NO ACTION OR NON-WARRANTED
001	SKIDDED	SKIDDED
002	ON/OFF, V	GETTING ON OR OFF STOPPED OR PARKED VEHICLE
003	LOAD OVR	OVERHANGING LOAD STRUCK ANOTHER VEHICLE, ETC.
006	SLOW DN	SLOWED DOWN
007	AVOIDING	AVOIDING MANEUVER
008	PARK PARK	PARALLEL PARKING
009	ANG PARK	ANGLE PARKING
010	INTERFERE	PASSENGER INTERFERING WITH DRIVER
011	STOPPED	STOPPED IN TRAFFIC NOT WAITING TO MAKE A LEFT TURN
012	STP/L TRN	STOPPED BECAUSE OF LEFT TURN SIGNAL OR WAITING, ETC.
013	STP TURN	STOPPED WHILE EXECUTING A TURN
015	GO A/STOP	PROCEEDED AFTER STOPPING FOR A STOP SIGN/FLASHING RED.
016	TRN A/RED	TURNED ON RED AFTER STOPPING
017	LOST CTRL	LOST CONTROL OF VEHICLE
018	EXIT DWY	ENTERING STREET OR HIGHWAY FROM ALLEY OR DRIVEWAY
019	ENTR DWY	ENTERING ALLEY OR DRIVEWAY FROM STREET OR HIGHWAY
020	STR ENTR	BEFORE ENTERING ROADWAY, STRUCK PEDESTRIAN, ETC. ON SIDEWALK OR SHOULDER
021	NO DRV'R	CAR RAN AWAY - NO DRIVER
022	PREV COL	STRUCK, OR WAS STRUCK BY, VEHICLE OR PEDESTRIAN IN PRIOR COLLISION BEFORE ACC. STABILIZED
023	STALLED	VEHICLE STALLED OR DISABLED
024	DRV'R DEAD	DEAD BY UNASSOCIATED CAUSE
025	FATIGUE	FATIGUED, SLEEPY, ASLEEP
026	SUN	DRIVER BLINDED BY SUN
027	HDLIGHTS	DRIVER BLINDED BY HEADLIGHTS
028	ILLNESS	PHYSICALLY ILL
029	THRU MED	VEHICLE CROSSED, PLUNGED OVER, OR THROUGH MEDIAN BARRIER
030	PURSUIT	PURSUING OR ATTEMPTING TO STOP A VEHICLE
031	PASSING	PASSING SITUATION
032	PRKFORD	VEHICLE PARKED BEYOND CURB OR SHOULDER
033	CROS MED	VEHICLE CROSSED EARLY OR GRASS MEDIAN
034	X N/SGNL	CROSSING AT INTERSECTION - NO TRAFFIC SIGNAL PRESENT
035	X W/ SGNL	CROSSING AT INTERSECTION - TRAFFIC SIGNAL PRESENT
036	DIAGONAL	CROSSING AT INTERSECTION - DIAGONALLY
037	BTWN INT	CROSSING BETWEEN INTERSECTIONS
038	DISTRACT	DRIVER'S ATTENTION DISTRACTED
039	W/TRAFF-S	WALKING, RUNNING, RIDING, ETC., ON SHOULDER WITH TRAFFIC
040	A/TRAFF-S	WALKING, RUNNING, RIDING, ETC., ON SHOULDER FACING TRAFFIC
041	W/TRAFF-P	WALKING, RUNNING, RIDING, ETC., ON PAVEMENT WITH TRAFFIC
042	A/TRAFF-P	WALKING, RUNNING, RIDING, ETC., ON PAVEMENT FACING TRAFFIC
043	PLAY/IRD	PLAYING IN STREET OR ROAD
044	PUSH MV	PUSHING OR WORKING ON VEHICLE IN ROAD OR ON SHOULDER
045	WORK ON	WORKING IN ROADWAY OR ALONG SHOULDER
046	W/ TRAFFIC	NON-MOTORIST WALKING, RUNNING, RIDING, ETC. WITH TRAFFIC
047	A/ TRAFFIC	NON-MOTORIST WALKING, RUNNING, RIDING, ETC. FACING TRAFFIC
050	LAY ON RD	STANDING OR LYING IN ROADWAY
051	ENT OFRD	ENTERING / STARTING IN TRAFFIC LANE FROM OFF ROAD
052	MERGING	MERGING
055	SPRAY	BLINDED BY WATER SPRAY
088	OTHER	OTHER ACTION

ACTION CODE TRANSLATION LIST

ACTION CODE	SHORT DESCRIPTION	LONG DESCRIPTION
099	UNK	UNKNOWN ACTION

CAUSE CODE TRANSLATION LIST

CAUSE CODE	SHORT DESCRIPTION	LONG DESCRIPTION
00	NO CODE	NO CAUSE ASSOCIATED AT THIS LEVEL
01	TOO-FAST	TOO FAST FOR CONDITIONS (NOT EXCEED POSTED SPEED)
02	NO-YIELD	DID NOT YIELD RIGHT-OF-WAY
03	PAS-STOP	PASSED STOP SIGN OR RED FLASHER
04	DIS SIG	DISREGARDED TRAFFIC SIGNAL
05	LEFT-CTR	DOVE LEFT OF CENTER ON TWO-WAY ROAD; STRADDLING
06	IMP-OVER	IMPROPER OVERTAKING
07	TOO-CLOS	FOLLOWED TOO CLOSELY
08	IMP-TURN	MADE IMPROPER TURN
09	DRINKING	ALCOHOL OR DRUG INVOLVED
10	OTHR-TMP	OTHER IMPROPER DRIVING
11	MECH-DEF	MECHANICAL DEFECT
12	OTHER	OTHER (NOT IMPROPER DRIVING)
13	IMP LN C	IMPROPER CHANGE OF TRAFFIC LANES
14	DIS TCD	DISREGARDED OTHER TRAFFIC CONTROL DEVICE
15	WRNG WAY	WRONG WAY ON ONE-WAY ROAD; WRONG SIDE DIVIDED RO.
16	FATIGUE	DRIVER DROWSY/FATIGUED/SLEEPY
17	ILLNESS	PHYSICAL ILLNESS
18	IN RDWY	NON-MOTORIST ILLEGALLY IN ROADWAY
19	NT VLSBL	NON-MOTORIST NOT VISIBLE; NON-REFLECTIVE CLOTHIN
20	IMP PKNG	VEHICLE IMPROPERLY PARKED
21	DEF STER	DEFECTIVE STEERING MECHANISM
22	DEF BRKE	INADEQUATE OR NO BRAKES
24	LOADSHIFT	VEHICLE LOST LOAD OR LOAD SHIFTED
25	TIREFAIL	TIRE FAILURE
26	PHANTOM	PHANTOM / NON-CONTACT VEHICLE
27	INATTENT	INATTENTION
28	NM INATT	NON-MOTORIST INATTENTION
29	F AVOID	FAILED TO AVOID VEHICLE AHEAD
30	SPEED	DRIVING IN EXCESS OF POSTED SPEED
31	RACING	SPEED RACING (PER PAR)
32	CARELESS	CARELESS DRIVING (PER PAR)
33	RECKLESS	RECKLESS DRIVING (PER PAR)
34	AGGRESV	AGGRESSIVE DRIVING (PER PAR)
35	RD RAGE	ROAD RAGE (PER PAR)
40	VIEW OBS	VIEW OBSCURED
50	USED MDN	IMPROPER USE OF MEDIAN OR SHOULDER

CRASH TYPE CODE TRANSLATION LIST

CRASH TYPE	SHORT DESCRIPTION	LONG DESCRIPTION
&	OVERTURN	OVERTURNED
0	NON-COLL	OTHER NON-COLLISION
1	OTH RDWY	MOTOR VEHICLE ON OTHER ROADWAY
2	PRKD MV	PARKED MOTOR VEHICLE
3	PED	PEDESTRIAN
4	TRAIN	RAILWAY TRAIN
6	BIKE	PEDALCYCLIST
7	ANIMAL	ANIMAL
8	FIX OBJ	FIXED OBJECT
9	OTH OBJ	OTHER OBJECT
A	ANGL-STP	ENTERING AT ANGLE - ONE VEHICLE STOPPED
B	ANGL-OTH	ENTERING AT ANGLE - ALL OTHERS
C	S-STRAIGHT	FROM SAME DIRECTION - BOTH GOING STRAIGHT
D	S-1TURN	FROM SAME DIRECTION - ONE TURN, ONE STRAIGHT
E	S-1STOP	FROM SAME DIRECTION - ONE STOPPED
F	S-OTHER	FROM SAME DIRECTION-ALL OTHERS, INCLUDING PARKING
G	O-STRAIGHT	FROM OPPOSITE DIRECTION - BOTH GOING STRAIGHT
H	O-1 L-TURN	FROM OPPOSITE DIRECTION-ONE LEFT TURN, ONE STRAIGHT
I	O-1STOP	FROM OPPOSITE DIRECTION - ONE STOPPED
J	O-OTHER	FROM OPPOSITE DIRECTION-ALL OTHERS INCL. PARKING

DRIVER LICENSE CODE TRANSLATION LIST

LIC CODE	SHORT DESC	LONG DESCRIPTION
0	NONE	NOT LICENSED (HAD NEVER BEEN LICENSED)
1	OR-Y	VALID OREGON LICENSE
2	OTH-Y	VALID LICENSE, OTHER STATE OR COUNTRY
3	SUSP	SUSPENDED/REVOKED

DRIVER RESIDENCE CODE TRANSLATION LIST

RES CODE	SHORT DESC	LONG DESCRIPTION
1	OR-25	OREGON RESIDENT WITHIN 25 MILE OF HOME
2	OR>25	OREGON RESIDENT 25 OR MORE MILES FROM HOME
3	OR=?	OREGON RESIDENT - UNKNOWN DISTANCE FROM HOME
4	N-RES	NON-RESIDENT
9	UNK	UNKNOWN IF OREGON RESIDENT

ERROR CODE TRANSLATION LIST

ERROR CODE	SHORT DESCRIPTION	FULL DESCRIPTION
000	NONE	NO ERROR
001	WIDE TRN	WIDE TURN
002	CUT CORN	CUT CORNER ON TURN
003	FAIL TRN	FAILED TO OBEY MANDATORY TRAFFIC TURN SIGNAL, SIGN OR LANE MARKINGS
004	L IN TRN	LEFT TURN IN FRONT OF ONCOMING TRAFFIC
005	L PROHIB	LEFT TURN WHERE PROHIBITED
006	FRM WRNG	TURNED FROM WRONG LANE
007	TO WRONG	TURNED INTO WRONG LANE
008	ILLEG U	U-TURNED ILLEGALLY
009	IMP STOP	IMPROPERLY STOPPED IN TRAFFIC LANE
010	IMP SIG	IMPROPER SIGNAL OR FAILURE TO SIGNAL
011	IMP BACK	BACKING IMPROPERLY (NOT PARKING)
012	IMP PARK	IMPROPERLY PARKED
013	UNPARK	IMPROPER START LEAVING PARKED POSITION
014	IMP STRT	IMPROPER START FROM STOPPED POSITION
015	IMP LGHT	IMPROPER OR NO LIGHTS (VEHICLE IN TRAFFIC)
016	INATTEN	INATTENTION (FAILURE TO DIM LIGHTS PRIOR TO 4/1/97)
017	UNSF VEH	DRIVING UNSAFE VEHICLE (NO OTHER ERROR APPARENT)
018	OTH PARK	ENTERING/EXITING PARKED POSITION W/ INSUFFICIENT CLEARANCE; OTHER IMPROPER PARKING MANEUVER
019	DIS DRV	DISREGARDED OTHER DRIVER'S SIGNAL
020	DIS SGNL	DISREGARDED TRAFFIC SIGNAL
021	RAN STOP	DISREGARDED STOP SIGN OR FLASHING RED
022	DIS SIGN	DISREGARDED WARNING SIGN, FLARES OR FLASHING AMBER
023	DIS OFCR	DISREGARDED POLICE OFFICER OR FLAGMAN
024	DIS EMER	DISREGARDED SIREN OR WARNING OF EMERGENCY VEHICLE
025	DIS RR	DISREGARDED RR SIGNAL, RR SIGN, OR RR FLAGMAN
026	REAR-END	FAILED TO AVOID STOPPED OR PARKED VEHICLE AHEAD OTHER THAN SCHOOL BUS
027	BIKE ROW	DID NOT HAVE RIGHT-OF-WAY
028	NO ROW	DID NOT HAVE RIGHT-OF-WAY
029	PED ROW	FAILED TO YIELD RIGHT-OF-WAY TO PEDESTRIAN
030	PAS CURV	PASSING ON THE WRONG SIDE
031	PAS WRNG	PASSING ON STRAIGHT ROAD UNDER UNSAFE CONDITIONS
032	PAS TANG	PASSED VEHICLE STOPPED AT CROSSWALK FOR PEDESTRIAN
033	PAS X-WK	PASSING AT INTERSECTION
034	PAS INTR	PASSING ON CREST OF HILL
035	PAS HILL	PASSING IN "NO PASSING" ZONE
036	N/PAS ZN	PASSING IN FRONT OF ONCOMING TRAFFIC
037	PAS TRAF	CUTTING IN (TWO LANES - TWO WAY ONLY)
038	CUT-IN	DRIVING ON WRONG SIDE OF THE ROAD (2-WAY UNDIVIDED ROADWAYS)
039	WRNSIDE	DRIVING THROUGH SAFETY ZONE OR OVER ISLAND
040	THRU MED	FAILED TO STOP FOR SCHOOL BUS
041	F/ST BUS	

ERROR CODE TRANSLATION LIST

ERROR CODE	SHORT DESCRIPTION	FULL DESCRIPTION
042	F/SLO MY	FAILED TO DECREASE SPEED FOR SLOWER MOVING VEHICLE
043	TOO CLOSE	FOLLOWING TOO CLOSELY (MUST BE ON OFFICER'S REPORT)
044	STROLL LN	STRADDLING OR DRIVING ON WRONG LANES
045	IMP CHG	IMPROPER CHANGE OF TRAFFIC LANES
046	WRNG WAY	WRONG WAY ON ONE-WAY ROADWAY; WRONG SIDE DIVIDED ROAD
047	BASCRLBE	DRIVING TOO FAST FOR CONDITIONS (NOT EXCEEDING POSTED SPEED)
048	OPEN DOOR	OPENED DOOR INTO ADJACENT TRAFFIC LANE
049	IMPEDING	IMPEDING TRAFFIC
050	SPEED	DRIVING IN EXCESS OF POSTED SPEED
051	RECKLESS	RECKLESS DRIVING (PER PAR)
052	CARELESS	CARELESS DRIVING (PER PAR)
053	RACING	SPEED RACING (PER PAR)
054	X N/SGNL	CROSSING AT INTERSECTION, NO TRAFFIC SIGNAL PRESENT
055	X W/SGNL	CROSSING AT INTERSECTION, TRAFFIC SIGNAL PRESENT
056	DIAGONAL	CROSSING AT INTERSECTION - DIAGONALLY
057	BTWN INT:	CROSSING BETWEEN INTERSECTIONS
059	W/TRAFF-S	WALKING, RUNNING, RIDING, ETC., ON SHOULDER WITH TRAFFIC
060	A/TRAFF-S	WALKING, RUNNING, RIDING, ETC., ON SHOULDER FACING TRAFFIC
061	W/TRAFF-P	WALKING, RUNNING, RIDING, ETC., ON PAVEMENT WITH TRAFFIC
062	A/TRAFF-P	WALKING, RUNNING, RIDING, ETC., ON PAVEMENT FACING TRAFFIC
063	PLAYIN RD	PLAYING IN STREET OR ROAD
064	PUSH MV	PUSHING OR WORKING ON VEHICLE IN ROAD OR ON SHOULDER
065	WORK IN RD	WORKING IN ROADWAY OR ALONG SHOULDER
070	LAY ON RD	STANDING OR LYING IN ROADWAY
071	NM IMP USE	IMPROPER USE OF TRAFFIC LANE BY NON-MOTORIST
073	ELUDING	ELUDING / ATTEMPT TO ELUDE
079	F NEG CURV	FAILED TO NEGOTIATE A CURVE
080	FAIL LN	FAILED TO MAINTAIN LANE
081	OFF RD	RAN OFF ROAD
082	NO CLEAR	DRIVER MISJUDGED CLEARANCE
083	OVERSTEER	OVER-CORRECTING
084	NOT USED	CODE NOT IN USE
085	OVRLOAD	OVERRADING OR IMPROPER LOADING OF VEHICLE WITH CARGO OR PASSENGERS
097	UNA DIS TC	UNABLE TO DETERMINE WHICH DRIVER DISREGARDED TRAFFIC CONTROL DEVICE

EVENT CODE TRANSLATION LIST

EVENT CODE	SHORT DESCRIPTION	LONG DESCRIPTION
001	FELL/JUMP	OCCUPANT FELL, JUMPED OR WAS EJECTED FROM MOVING VEHICLE
002	INTERFER	PASSENGER INTERFERED WITH DRIVER
003	BUG INTE	ANIMAL OR INSECT IN VEHICLE INTERFERED WITH DRIVER
004	INDRCT PED	PEDESTRIAN INDIRECTLY INVOLVED (NOT STRUCK)
005	SUB-PED	"SUB-PED": PEDESTRIAN INJURED SUBSEQUENT TO COLLISION, ETC.
006	INDRCT BIK	PEDALCYCLIST INDIRECTLY INVOLVED (NOT STRUCK)
007	HITCHHIKR	HITCHHAKER (SOLICITING A RIDE)
008	PSNGR TOW	PASSENGER OR NON-MOTORIST BEING TOWED OR PUSHED ON CONVEYANCE
009	ON/OFF V	GETTING ON/OFF STOPPED/PARKED VEHICLE (OCCUPANTS ONLY; MUST HAVE PHYSICAL CONTACT W/ VEHIC
010	SUB OTRN	SUB-OVERTURNED AFTER FIRST HARMFUL EVENT
011	MV PUSHD	VEHICLE BEING PUSHED
012	MV TOWED	VEHICLE TOWED OR HAD BEEN TOWING ANOTHER VEHICLE
013	FORCED	VEHICLE FORCED BY IMPACT INTO ANOTHER VEHICLE, PEDALCYCLIST OR PEDESTRIAN
014	SET MOTN	VEHICLE SET IN MOTION BY NON-DRIVER (CHILD RELEASED BRAKES, ETC.)
015	RR ROW	AT OR ON RAILROAD RIGHT-OF-WAY (NOT LIGHT RAIL)
016	LT RL ROW	AT OR ON LIGHT-RAIL RIGHT-OF-WAY (NOT LIGHT RAIL)
017	RR HIT V	TRAIN STRUCK VEHICLE
018	V HIT RR	VEHICLE STRUCK RAILROAD TRAIN
019	HIT RR CAR	VEHICLE STRUCK RAILROAD CAR ON ROADWAY
020	JACKNIFE	JACKNIFE; TRAILER OR TOWED VEHICLE STRUCK TOWING VEHICLE
021	TRL OTRN	TRAILER OR TOWED VEHICLE OVERTURNED
022	CN BROKE	TRAILER CONNECTION BROKE
023	DETACH TRL	DETACHED TRAILING OBJECT STRUCK OTHER VEHICLE, NON-MOTORIST, OR OBJECT
024	V DOOR OPEN	VEHICLE DOOR OPENED INTO ADJACENT TRAFFIC LANE
025	WHEEL OFF	WHEEL CAME OFF
026	HOOD UP	HOOD FLEW UP
028	LOAD SHIFT	LOAD LOAD, LOAD MOVED OR SHIFTED
029	TIREFAIL	TIRE FAILURE
030	PET	PET: CAT, DOG AND SIMILAR
031	LVSTOCK	STOCK: COW, CALF, BULL, STEER, SHEEP, ETC.
032	HORSE	HORSE, MULE, OR DONKEY
033	HRSE&RID	HORSE AND RIDER
034	GAME	WILD ANIMAL, GAME (INCLUDES BIRDS; NOT DEER OR ELK)
035	DEER ELK	DEER OR ELK, WAPITI
036	ANML VEH	ANIMAL-DRAWN VEHICLE
037	CULVERT	CULVERT, OPEN LOW OR HIGH MANHOLE
038	ATTENATN	IMPACT ATTENUATOR
039	PK METER	PARKING METER
040	CURB	CURB (ALSO NARROW SIDEWALKS ON BRIDGES)
041	JIGGLE	JIGGLE BAR OR TRAFFIC SNAKE FOR CHANNELIZATION
042	GDRL END	LEADING EDGE OF GUARDRAIL
043	GARDRAIL	GUARD RAIL (NOT METAL MEDIAN BARRIER)
044	BARRIER	MEDIAN BARRIER (RAISED OR METAL)
045	WALL	RETAINING WALL OR TUNNEL WALL
046	BR RAIL	BRIDGE RAILING OR PARAPET (ON BRIDGE OR APPROACH)
047	BR ABUTMT	BRIDGE ABUTMENT (INCLUDED "APPROACH END" THRU 2013)
048	BR COLNN	BRIDGE PILLAR OR COLUMN
049	BR GIRDR	BRIDGE GIRDER (HORIZONTAL BRIDGE STRUCTURE OVERHEAD)
050	ISLAND	TRAFFIC RAISED ISLAND
051	GORE	GORE
052	POLE UNK	POLE - TYPE UNKNOWN
053	POLE UTL	POLE - POWER OR TELEPHONE
054	ST LIGHT	POLE - STREET LIGHT ONLY
055	TRF SGNL	POLE - TRAFFIC SIGNAL AND PED SIGNAL ONLY
056	SGN BRDG	POLE - SIGN BRIDGE
057	STOP/SIGN	STOP OR YIELD SIGN
058	OTH SIGN	OTHER SIGN, INCLUDING STREET SIGNS
059	HYDRANT	HYDRANT

EVENT CODE TRANSLATION LIST

EVENT CODE	SHORT DESCRIPTION	LONG DESCRIPTION
060	MARKER	DELINERATOR OR MARKER (REFLECTOR POSTS)
061	MAILBOX	MAILBOX
062	TREE, STUMP OR SHRUBS	TREE, BRANCH OR OTHER VEGETATION OVERHEAD, ETC.
063	VEG OHED	WIRE OR CABLE ACROSS OR OVER THE ROAD
064	WIRE/CBL	TEMPORARY SIGN OR BARRICADE IN ROAD, ETC.
065	TEMP SGN	PERMANENT SIGN OR BARRICADE IN/OUT ROAD
066	PERM SGN	SLIDES, FALLEN OR FALLING ROCKS
067	SLIDS	FOREIGN OBSTRUCTION/DEBRIS IN ROAD (NOT GRAVEL)
068	FRGN OBJ	EQUIPMENT WORKING IN/OUT ROAD
069	EQP WORK	OTHER EQUIPMENT IN OR OFF ROAD (INCLUDES PARKED TRAILER, BOAT)
070	OTH EQP	WECKER, STREET SWEeper, SNOW PLOW OR SANDING EQUIPMENT
071	MAIN EQP	ROCK, BRICK OR OTHER SOLID WALL
072	OTHER WALL	OTHER BUMP (NOT SPEED BUMP), POTHOLE OR PAVEMENT IRRREGULARITY (PER PAR)
073	IRGL PYNT	OTHER OVERHEAD OBJECT (HIGHWAY SIGN, SIGNAL HEAD, ETC.); NOT BRIDGE
074	OVERHD OBJ	BRIDGE OR ROAD CAVE IN
075	CAVE IN	HIGH WATER
076	HI WATER	SNO BANK
077	HIGH WATER	SNOW BANK
078	LO-HI EDGE	LOW OR HIGH SHOULDER AT PAVEMENT EDGE
079	DITCH	CUT SLOPE OR DITCH EMBANKMENT
080	OBJ FRM MV	STRUCK BY ROCK OR OTHER OBJECT SET IN MOTION BY OTHER VEHICLE (INCL. LOST LOADS)
081	FLY-OBJ	STRUCK BY ROCK OR OTHER MOVING OR FLYING OBJECT (NOT SET IN MOTION BY VEHICLE)
082	VEH HID	VEHICLE OBSCURED VIEW
083	VEG HID	VEGETATION OBSCURED VIEW
084	BLDG HID	VIEW OBSCURED BY FENCE, SIGN, PHONE BOOTH, ETC.
085	WIND GUST	WIND GUST
086	IMMERSED	VEHICLE IMMERSED IN BODY OF WATER
087	FIRE/EXP	FIRE OR EXPLOSION
088	FENC/BLD	FENCE OR BUILDING, ETC.
089	OTHR CRASH	CRASH RELATED TO ANOTHER SEPARATE CRASH
090	TO 1 SIDE	TWO-WAY TRAFFIC ON DIVIDED ROADWAY ALL ROUTED TO ONE SIDE
091	BUILDING	BUILDING OR OTHER STRUCTURE
092	PHANTOM	OTHER (PHANTOM) NON-CONTACT VEHICLE
093	CELL PHONE	CELL PHONE (ON PAR OR DRIVER IN USE)
094	VIOL GDL	TEENAGE DRIVER IN VIOLATION OF GRADUATED LICENSE PGM
095	GUY WIRE	GUY WIRE
096	BERM	BERM (EARTHEN OR GRAVEL MOUND)
097	GRAVEL	GRAVEL IN ROADWAY
098	ABR EDGE	ABRUPT EDGE
099	CELL WTNSD	CELL PHONE USE WITNESSED BY OTHER PARTICIPANT
100	UNK FIXD	FIXED OBJECT, UNKNOWN TYPE.
101	OTHER OBJ	NON-FIXED OBJECT, OTHER OR UNKNOWN TYPE
102	TEXTING	TEXTING
103	WZ WORKER	WORK ZONE WORKER
104	ON VEHICLE	PASSENGER RIDING ON VEHICLE EXTERIOR
105	PEDAL PSGR	PASSENGER RIDING ON PEDALCYCLE
106	MAN WHLCHR	PEDESTRIAN IN NON-MOTORIZED WHEELCHAIR
107	MTR WHLCHR	PEDESTRIAN IN MOTORIZED WHEELCHAIR
108	OFFICER	LAW ENFORCEMENT / POLICE OFFICER
109	SUB-BIKE	"SUB-BIKE": PEDALCYCLIST INJURED SUBSEQUENT TO COLLISION, ETC.
110	N-MTR	NON-MOTORIST STRUCK VEHICLE
111	S CAR VS V	STREET CAR/TROLLEY (ON RAILS OR OVERHEAD WIRE SYSTEM) STRUCK VEHICLE
112	V VS S CAR	VEHICLE STRUCK STREET CART/TROLLEY (ON RAILS OR OVERHEAD WIRE SYSTEM)
113	S CAR ROW	AT OR ON STREET CAR OR TROLLEY RIGHT-OF-WAY
114	RR EQUIP	VEHICLE STRUCK RAILROAD EQUIPMENT (NOT TRAIN) ON TRACKS
115	DSTRCT GPS	DISTRCT BY NAVIGATION SYSTEM OR GPS DEVICE
116	DSTRCT OTH	Distracted by other electronic device
117	RR GATE	RAIL CROSSING DROP-ARM GATE

EVENT CODE TRANSLATION LIST

EVENT CODE	SHORT DESCRIPTION	LONG DESCRIPTION
118	EXPNSN JNT	EXPANSION JOINT
119	JERSEY BAR	JERSEY BARRIER
120	WIRE BAR	WIRE OR CABLE MEDIAN BARRIER
121	FENCE	FENCE
123	OBJ IN VEH	LOSE OBJECT IN VEHICLE STRUCK OCCUPANT
124	SLIPPERY	SLIDING OR SWERVING DUE TO WET, ICY, SLIPPERY OR LOOSE SURFACE (NOT GRAVEL)
125	SHLDR	SHOULDER GAVE WAY
126	BOULDER	ROCK (S), BOULDER (NOT GRAVEL; NOT ROCK SLIDE)
127	LAND SLIDE	ROCK SLIDE OR LAND SLIDE
128	CURVE INV	CURVE PRESENT AT CRASH LOCATION
129	HILL INV	VERTICAL GRADE / HILL PRESENT AT CRASH LOCATION
130	CURVE HID	VIEW OBSCURED BY CURVE
131	HILL HID	VIEW OBSCURED BY VERTICAL GRADE / HILL
132	WINDOW HID	VIEW OBSCURED BY VEHICLE WINDOW CONDITIONS
133	SPRAY HID	VIEW OBSCURED BY WATER SPRAY

FUNCTIONAL CLASSIFICATION TRANSLATION LIST

HIGHWAY COMPONENT TRANSLATION LIST

FUNC CLASS	DESCRIPTION	CODE	DESCRIPTION
01	RURAL PRINCIPAL ARTERIAL - INTERSTATE	0	MAINLINE STATE HIGHWAY
02	RURAL PRINCIPAL ARTERIAL - OTHER	1	COUPLER
06	RURAL MINOR ARTERIAL	3	FRONTAGE ROAD
07	RURAL MAJOR COLLECTOR	6	CONNECTION
08	RURAL MINOR COLLECTOR	8	HIGHWAY - OTHER
09	RURAL LOCAL		
11	URBAN PRINCIPAL ARTERIAL - INTERSTATE		
12	URBAN PRINCIPAL ARTERIAL - OTHER FREEWAYS AND EXP		
14	URBAN PRINCIPAL ARTERIAL - OTHER		
16	URBAN MINOR ARTERIAL		
17	URBAN MAJOR COLLECTOR		
18	URBAN MINOR COLLECTOR		
19	URBAN LOCAL		
78	UNKNOWN RURAL SYSTEM		
79	UNKNOWN RURAL NON-SYSTEM		
98	UNKNOWN URBAN SYSTEM		
99	UNKNOWN URBAN NON-SYSTEM		

INJURY SEVERITY CODE TRANSLATION LIST

SHORT CODE	LONG DESCRIPTION
1	KILL
2	FATAL INJURY
3	INCAPACITATING INJURY - BLEEDING, BROKEN BONES
4	NON-INCAPACITATING INJURY
5	POSSIBLE INJURY - COMPLAINT OF PAIN
6	DIED PRIOR TO CRASH
7	NO INJURY - 0 TO 4 YEARS OF AGE

LIGHT CONDITION CODE TRANSLATION LIST

SHORT CODE	LONG DESCRIPTION
0	UNKNOWN
1	DAY
2	DAWN (TWILIGHT)
3	DUSK (TWILIGHT)
4	DARK
5	DUSK

MEDIAN TYPE CODE TRANSLATION LIST

SHORT CODE	LONG DESCRIPTION
0	NONE
1	RSMD SOLID MEDIAN BARRIER
2	DIVMD EARTH, GRASS OR PAVED MEDIAN

MILEAGE TYPE CODE TRANSLATION LIST

CODE	LONG DESCRIPTION
0	REGULAR MILEAGE
1	TEMPORARY
2	SPUR
3	OVERLAPPING

MOVEMENT TYPE CODE TRANSLATION LIST

CODE	SHORT DESC	LONG DESCRIPTION
0	UNK	UNKNOWN
1	STRIGHT	STRAIGHT AHEAD
2	TURN-R	TURNING RIGHT
3	TURN-L	TURNING LEFT
4	U-TURN	MAKING A U-TURN
5	BACK	BACKING
6	STOP	STOPPED IN TRAFFIC
7	PRKD-P	PARKED - PROPERLY
8	PRKD-I	PARKED - IMPROPERLY

PEDESTRIAN LOCATION CODE TRANSLATION LIST

CODE	LONG DESCRIPTION
00	AT INTERSECTION - NOT IN ROADWAY
01	AT INTERSECTION - INSIDE CROSSWALK
02	AT INTERSECTION - IN ROADWAY, OUTSIDE CROSSWALK
03	AT INTERSECTION - IN ROADWAY, XWALK AVAIL UNKNOWN
04	NOT AT INTERSECTION - IN ROADWAY
05	NOT AT INTERSECTION - ON SHOULDER
06	NOT AT INTERSECTION - ON MEDIAN
07	NOT AT INTERSECTION - WITHIN TRAFFIC RIGHT-OF-WAY
08	NOT AT INTERSECTION - IN BIKE PATH OR PARKING LANE
09	NOT AT INTERSECTION - ON SIDEWALK
10	OUTSIDE TRAFFICWAY BOUNDARIES
11	AT INTERSECTION - IN BIKE LANE
12	NOT AT INTERSECTION - IN BIKE LANE
13	NOT AT INTERSECTION - INSIDE MID-BLOCK CROSSWALK
14	NOT AT INTERSECTION - INSIDE MID-BLOCK CROSSWALK
15	NOT AT INTERSECTION - IN PARKING LANE
16	NOT AT INTERSECTION - IN PARKING LANE

ROAD CHARACTER CODE TRANSLATION LIST

CODE	SHORT DESC	LONG DESCRIPTION
0	UNK	UNKNOWN
1	INTER	INTERSECTION
2	ALLEY	DRIVEWAY OR ALLEY
3	STRIGHT	STRAIGHT ROADWAY
4	TRANS	TRANSITION
5	CURVE	CURVE (HORIZONTAL CURVE)
6	OPENAC	OPEN ACCESS OR TURNOUT
7	GRADE	GRADE (VERTICAL CURVE)
8	BRIDGE	BRIDGE STRUCTURE
9	TUNNEL	TUNNEL

PARTICIPANT TYPE CODE TRANSLATION LIST

CODE	SHORT DESC	LONG DESCRIPTION
0	OCC	UNKNOWN OCCUPANT TYPE
1	DRV	DRIVER
2	PSNG	PASSENGER
3	PED	PEDESTRIAN
4	CONV	PEDESTRIAN USING A PEDESTRIAN CONVEYA
5	PTOW	PEDESTRIAN TOWING OR TRAILERING AN OB.
6	BIKE	PEDALCYCLIST
7	BTOW	PEDALCYCLIST TOWING OR TRAILERING AN OCCUPANT OF A PARKED MOTOR VEHICLE
8	PRKD	UNKNOWN TYPE OF NON-MOTORIST
9	UNK	UNKNOWN

TRAFFIC CONTROL DEVICE CODE TRANSLATION LIST

CODE	SHORT DESC	LONG DESCRIPTION
000	NONE	NO CONTROL
001	TRF SIGNAL	TRAFFIC SIGNALS
002	FLASHBGN-R	FLASHING BEACON - RED (STOP)
003	FLASHBGN-A	FLASHING BEACON - AMBER (SLOW)
004	STOP SIGN	STOP SIGN
005	SLOW SIGN	SLOW SIGN
006	REG-SIGN	REGULATORY SIGN
007	YIELD	YIELD SIGN
008	WARNING	WARNING SIGN
009	CURVE	CURVE SIGN
010	SCHL X-ING	SCHOOL CROSSING SIGN OR SPECIAL SIGNAL
011	OFCR/FLAG	POLICE OFFICER, FLAGMAN - SCHOOL PATROL
012	BRDG-GATE	BRIDGE GATE - BARRIER
013	TEMP-BARR	TEMPORARY BARRIER
014	NO-PASS-ZN	NO PASSING ZONE
015	ONE-WAY	ONE-WAY STREET
016	CHANNEL	CHANNELIZATION
017	MEDIAN BAR	MEDIAN BARRIER
018	PILOT CAR	PILOT CAR
019	SP PED SIG	SPECIAL PEDESTRIAN SIGNAL
020	X-BUCK	CROSSBUCK
021	THR-GN-SIG	THROUGH GREEN ARROW OR SIGNAL
022	L-GRN-SIG	LEFT TURN GREEN ARROW, LANE MARKINGS, OR SIGNAL
023	R-GRN-SIG	RIGHT TURN GREEN ARROW, LANE MARKINGS, OR SIGNAL
024	WIGWAG	WIGWAG OR FLASHING LIGHTS W/O DROP-ARM GATE
025	X-BUCK WRN	CROSSBUCK AND ADVANCE WARNING
026	WW W/ GATE	FLASHING LIGHTS WITH DROP-ARM GATES
027	OVHD SGNL	SUPPLEMENTAL OVERHEAD SIGNAL (RR XING ONLY)
028	SP RR STOP	SPECIAL RR STOP SIGN
029	ILLUM GRD X	ILLUMINATED GRADE CROSSING
030	RAMP METER	METERED RAMPS
038	RUMBLE STR	RUMBLE STRIP
090	L-TURN REF	LEFT TURN REFUGE (WHEN REFUGE IS INVOLVED)
091	R-TURN ALL	RIGHT TURN AT ALL TIMES SIGN, ETC.
092	EMR SGN FL	EMERGENCY SIGNS OR FLARES
093	ACCEL LANE	ACCELERATION OR DECLERATION LANES
094	R-TURN PRO	RIGHT TURN PROHIBITED ON RED AFTER STOPPING

VEHICLE TYPE CODE TRANSLATION LIST

CODE	SHORT DESC	LONG DESCRIPTION
00	PDO	NOT COLLECTED FOR PDO CRASHES
01	PSNGR CAR	PASSENGER CAR, PICKUP, LIGHT DELIVERY, ETC.
02	BOBTAIL	TRUCK TRACTOR WITH NO TRAILERS (BOBTAIL)
03	FARM TRCTR	FARM TRACTOR OR SELF-PROPELLED FARM EQUIPMENT
04	SEMI TOW	TRUCK TRACTOR WITH TRAILER/MOBILE HOME IN TOW
05	TRUCK	TRUCK WITH NON-DETACHABLE BED, PANEL, ETC.
06	MOPED	MOPED, MINIBIKE, SEATED MOTOR SCOOTER, MOTOR BIKE
07	SCHL BUS	SCHOOL BUS (INCLUDES VAN)
08	OTH BUS	OTHER BUS
09	MTRCYCLE	MOTORCYCLE, DIRT BIKE
10	OTHER	OTHER: FORKLIFT, BACKHOE, ETC.
11	MOTRHOME	MOTORHOME
12	TROLLEY	MOTORIZED STREET CAR/TROLLEY (NO RAILS/WIRES)
13	ATV	ATV
14	MTRSCTR	MOTORIZED SCOOTER (STANDING)
15	SNOWMOBILE	SNOWMOBILE
99	UNKNOWN	UNKNOWN VEHICLE TYPE

WEATHER CONDITION CODE TRANSLATION LIST

CODE	SHORT DESC	LONG DESCRIPTION
0	UNK	UNKNOWN
1	CLR	CLEAR
2	CLD	CLOUDY
3	RAIN	RAIN
4	SLT	SLEET
5	FOG	FOG
6	SNOW	SNOW
7	DUST	DUST
8	SMOK	SMOKE
9	ASH	ASH

095 BUS STOP SIGN AND RED LIGHTS
099 UNKNOWN OR NOT DEFINITE

Appendix D Year 2018 Background Traffic Conditions

Queues

1: Parking Lot/SW Tile Flat Rd & SW Scholls Ferry Rd

01/19/2017



Lane Group	EBL	EBT	WBT	SBT
Lane Group Flow (vph)	4	604	947	326
v/c Ratio	0.05	0.49	0.80	0.81
Control Delay	51.8	8.6	18.4	47.7
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	51.8	8.6	18.4	47.7
Queue Length 50th (ft)	3	165	363	157
Queue Length 95th (ft)	14	235	#766	#330
Internal Link Dist (ft)		230	1105	390
Turn Bay Length (ft)	130			
Base Capacity (vph)	83	1341	1177	410
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.05	0.45	0.80	0.80

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis
1: Parking Lot/SW Tile Flat Rd & SW Scholls Ferry Rd

01/19/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑			↔		↔	↔			↔	
Traffic Volume (vph)	4	604	0	0	527	420	0	0	0	317	0	9
Future Volume (vph)	4	604	0	0	527	420	0	0	0	317	0	9
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	12	12	12	12	12	12	11	11	11	12	12	12
Grade (%)		4%			2%				0%		0%	
Total Lost time (s)	4.0	5.5			5.5					4.5		
Lane Util. Factor	1.00	1.00			1.00					1.00		
Fr _t	1.00	1.00			0.94					1.00		
Flt Protected	0.95	1.00			1.00					0.95		
Satd. Flow (prot)	1769	1724			1679					1691		
Flt Permitted	0.95	1.00			1.00					0.95		
Satd. Flow (perm)	1769	1724			1679					1691		
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	4	604	0	0	527	420	0	0	0	317	0	9
RTOR Reduction (vph)	0	0	0	0	15	0	0	0	0	0	68	0
Lane Group Flow (vph)	4	604	0	0	932	0	0	0	0	0	258	0
Heavy Vehicles (%)	0%	8%	0%	0%	8%	2%	0%	0%	0%	6%	0%	33%
Turn Type	Prot	NA			NA					Split	NA	
Protected Phases	5	2			6		8	8		4	4	
Permitted Phases					6							
Actuated Green, G (s)	0.9	78.5			73.6					20.9		
Effective Green, g (s)	0.9	78.5			73.6					20.9		
Actuated g/C Ratio	0.01	0.72			0.67					0.19		
Clearance Time (s)	4.0	5.5			5.5					4.5		
Vehicle Extension (s)	1.5	3.5			3.5					1.5		
Lane Grp Cap (vph)	14	1237			1129					323		
v/s Ratio Prot	0.00	c0.35			c0.56					c0.15		
v/s Ratio Perm												
v/c Ratio	0.29	0.49			0.83					0.80		
Uniform Delay, d1	53.9	6.7			13.2					42.2		
Progression Factor	1.00	1.00			1.00					1.00		
Incremental Delay, d2	4.1	0.4			5.2					12.1		
Delay (s)	58.0	7.1			18.4					54.4		
Level of Service	E	A			B					D		
Approach Delay (s)		7.4			18.4		0.0			54.4		
Approach LOS		A			B		A			D		
Intersection Summary												
HCM 2000 Control Delay		21.1			HCM 2000 Level of Service		C					
HCM 2000 Volume to Capacity ratio		0.86										
Actuated Cycle Length (s)		109.4			Sum of lost time (s)		18.0					
Intersection Capacity Utilization		79.8%			ICU Level of Service		D					
Analysis Period (min)		15										
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis
 2: SW Vandermost Rd/SW Strobel Rd & SW Scholls Ferry Rd

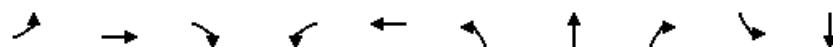
01/19/2017

Movement	EBL	EBT	EBC	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	910	0	5	954	0	0	0	3	0	0	0
Future Volume (Veh/h)	0	910	0	5	954	0	0	0	3	0	0	0
Sign Control	Free				Free			Stop			Stop	
Grade	-4%				0%			-7%			-4%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	0	910	0	5	954	0	0	0	3	0	0	0
Pedestrians	1											
Lane Width (ft)	12.0											
Walking Speed (ft/s)	3.5											
Percent Blockage	0											
Right turn flare (veh)												
Median type	None				None							
Median storage veh												
Upstream signal (ft)	1185											
pX, platoon unblocked					0.86			0.86	0.86	0.86	0.86	0.86
vC, conflicting volume	954				910			1875	1874	910	1877	1874
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	954				810			1939	1937	810	1941	1937
tC, single (s)	4.1				4.1			7.1	6.5	6.5	7.1	6.5
tC, 2 stage (s)												
tF (s)	2.2				2.2			3.5	4.0	3.6	3.5	4.0
p0 queue free %	100				99			100	100	99	100	100
cM capacity (veh/h)	729				705			43	57	288	42	57
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	910	959	3	0								
Volume Left	0	5	0	0								
Volume Right	0	0	3	0								
cSH	729	705	288	1700								
Volume to Capacity	0.00	0.01	0.01	0.00								
Queue Length 95th (ft)	0	1	1	0								
Control Delay (s)	0.0	0.2	17.6	0.0								
Lane LOS		A	C	A								
Approach Delay (s)	0.0	0.2	17.6	0.0								
Approach LOS		C	A									
Intersection Summary												
Average Delay			0.1									
Intersection Capacity Utilization		64.5%			ICU Level of Service				C			
Analysis Period (min)			15									

Queues

3: SW Roy Rogers Rd/SW 175th Ave & SW Scholls Ferry Rd

01/19/2017



Lane Group	EBL	EBT	EBC	WBL	WBT	NBL	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	34	486	433	453	589	421	495	639	103	562
v/c Ratio	0.37	0.78	0.56	0.83	0.51	0.90	0.71	0.66	0.71	0.83
Control Delay	66.4	52.8	16.9	60.3	31.8	62.4	35.7	16.2	77.5	54.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	66.4	52.8	16.9	60.3	31.8	62.4	35.7	16.2	77.5	54.5
Queue Length 50th (ft)	26	188	163	173	192	304	309	246	77	215
Queue Length 95th (ft)	62	251	255	#263	254	#514	464	413	#163	#308
Internal Link Dist (ft)		686			809		447			409
Turn Bay Length (ft)	170		330	330		300		300	250	
Base Capacity (vph)	109	835	844	635	1279	548	811	1004	178	796
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.31	0.58	0.51	0.71	0.46	0.77	0.61	0.64	0.58	0.71

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis
3: SW Roy Rogers Rd/SW 175th Ave & SW Scholls Ferry Rd

01/19/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑	↑	↑↑	↑↑		↑	↑	↑	↑	↑↑	
Traffic Volume (vph)	34	486	433	453	475	114	421	495	639	103	512	50
Future Volume (vph)	34	486	433	453	475	114	421	495	639	103	512	50
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)	0%			0%				3%			-2%	
Total Lost time (s)	4.0	5.5	4.0	4.0	5.5		4.0	5.5	4.0	4.0	5.5	
Lane Util. Factor	1.00	0.95	1.00	0.97	0.95		1.00	1.00	1.00	1.00	0.95	
Frpb, ped/bikes	1.00	1.00	0.99	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Frt	1.00	1.00	0.85	1.00	0.97		1.00	1.00	0.85	1.00	0.99	
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1736	3406	1515	3367	3410		1662	1782	1530	1770	3495	
Flt Permitted	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (perm)	1736	3406	1515	3367	3410		1662	1782	1530	1770	3495	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	34	486	433	453	475	114	421	495	639	103	512	50
RTOR Reduction (vph)	0	0	48	0	17	0	0	0	46	0	6	0
Lane Group Flow (vph)	34	486	385	453	572	0	421	495	593	103	556	0
Confl. Peds. (#/hr)							1				1	
Confl. Bikes (#/hr)			2									
Heavy Vehicles (%)	4%	6%	6%	4%	3%	2%	7%	5%	4%	3%	3%	1%
Turn Type	Prot	NA	pm+ov	Prot	NA		Prot	NA	pm+ov	Prot	NA	
Protected Phases	5	2	3	1	6		3	8	1	7	4	
Permitted Phases			2					8				
Actuated Green, G (s)	3.4	22.0	52.8	17.6	36.2		30.8	42.8	60.4	9.1	21.1	
Effective Green, g (s)	3.4	22.0	52.8	17.6	36.2		30.8	42.8	60.4	9.1	21.1	
Actuated g/C Ratio	0.03	0.20	0.48	0.16	0.33		0.28	0.39	0.55	0.08	0.19	
Clearance Time (s)	4.0	5.5	4.0	4.0	5.5		4.0	5.5	4.0	4.0	5.5	
Vehicle Extension (s)	1.0	2.0	1.0	1.0	2.0		1.0	2.0	1.0	1.0	2.0	
Lane Grp Cap (vph)	53	678	723	536	1117		463	690	836	145	667	
v/s Ratio Prot	0.02	c0.14	0.15	c0.13	0.17		c0.25	0.28	0.11	0.06	c0.16	
v/s Ratio Perm			0.11					0.27				
v/c Ratio	0.64	0.72	0.53	0.85	0.51		0.91	0.72	0.71	0.71	0.83	
Uniform Delay, d1	52.9	41.3	20.2	45.1	30.0		38.5	28.7	18.6	49.4	43.0	
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	18.1	3.0	0.4	11.2	0.2		21.1	3.0	2.3	12.8	8.5	
Delay (s)	71.1	44.4	20.6	56.4	30.2		59.6	31.7	20.8	62.2	51.5	
Level of Service	E	D	C	E	C		E	C	C	E	D	
Approach Delay (s)		34.5			41.6			34.8			53.1	
Approach LOS		C			D			C			D	
Intersection Summary												
HCM 2000 Control Delay		39.3										D
HCM 2000 Volume to Capacity ratio		0.83										
Actuated Cycle Length (s)		110.5										19.0
Intersection Capacity Utilization		81.4%										D
Analysis Period (min)		15										
c Critical Lane Group												

Queues

1: Parking Lot/SW Tile Flat Rd & SW Scholls Ferry Rd

01/18/2017



Lane Group	EBL	EBT	WBT	SBT
Lane Group Flow (vph)	4	671	1053	362
v/c Ratio	0.05	0.55	0.90	0.89
Control Delay	51.8	9.7	25.9	56.6
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	51.8	9.7	25.9	56.6
Queue Length 50th (ft)	3	195	475	186
Queue Length 95th (ft)	14	277	#1013	#393
Internal Link Dist (ft)		230	1105	390
Turn Bay Length (ft)	130			
Base Capacity (vph)	83	1333	1170	408
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.05	0.50	0.90	0.89

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis
1: Parking Lot/SW Tile Flat Rd & SW Scholls Ferry Rd

01/18/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑			↔		↔	↔			↔	
Traffic Volume (vph)	4	604	0	0	527	420	0	0	0	317	0	9
Future Volume (vph)	4	604	0	0	527	420	0	0	0	317	0	9
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	12	12	12	12	12	12	11	11	11	12	12	12
Grade (%)		4%			2%				0%		0%	
Total Lost time (s)	4.0	5.5			5.5					4.5		
Lane Util. Factor	1.00	1.00			1.00					1.00		
Fr _t	1.00	1.00			0.94					1.00		
Flt Protected	0.95	1.00			1.00					0.95		
Satd. Flow (prot)	1769	1724			1679					1691		
Flt Permitted	0.95	1.00			1.00					0.95		
Satd. Flow (perm)	1769	1724			1679					1691		
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	4	671	0	0	586	467	0	0	0	352	0	10
RTOR Reduction (vph)	0	0	0	0	15	0	0	0	0	0	68	0
Lane Group Flow (vph)	4	671	0	0	1038	0	0	0	0	0	294	0
Heavy Vehicles (%)	0%	8%	0%	0%	8%	2%	0%	0%	0%	6%	0%	33%
Turn Type	Prot	NA			NA					Split	NA	
Protected Phases	5	2			6		8	8		4	4	
Permitted Phases					6							
Actuated Green, G (s)	0.9	78.5			73.6					21.5		
Effective Green, g (s)	0.9	78.5			73.6					21.5		
Actuated g/C Ratio	0.01	0.71			0.67					0.20		
Clearance Time (s)	4.0	5.5			5.5					4.5		
Vehicle Extension (s)	1.5	3.5			3.5					1.5		
Lane Grp Cap (vph)	14	1230			1123					330		
v/s Ratio Prot	0.00	c0.39			c0.62					c0.17		
v/s Ratio Perm												
v/c Ratio	0.29	0.55			0.92					0.89		
Uniform Delay, d1	54.2	7.4			15.8					43.1		
Progression Factor	1.00	1.00			1.00					1.00		
Incremental Delay, d2	4.1	0.6			12.7					24.2		
Delay (s)	58.3	7.9			28.5					67.3		
Level of Service	E	A			C					E		
Approach Delay (s)		8.2			28.5		0.0			67.3		
Approach LOS		A			C		A			E		
Intersection Summary												
HCM 2000 Control Delay		28.7			HCM 2000 Level of Service					C		
HCM 2000 Volume to Capacity ratio		0.96										
Actuated Cycle Length (s)		110.0			Sum of lost time (s)					18.0		
Intersection Capacity Utilization		79.8%			ICU Level of Service					D		
Analysis Period (min)		15										
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis
 2: SW Vandermost Rd/SW Strobel Rd & SW Scholls Ferry Rd

01/18/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔			↔	
Traffic Volume (veh/h)	0	910	0	5	954	0	0	0	3	0	0	0
Future Volume (Veh/h)	0	910	0	5	954	0	0	0	3	0	0	0
Sign Control		Free			Free			Stop			Stop	
Grade		-4%			0%			-7%			-4%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	1011	0	6	1060	0	0	0	3	0	0	0
Pedestrians		1										
Lane Width (ft)		12.0										
Walking Speed (ft/s)		3.5										
Percent Blockage		0										
Right turn flare (veh)												
Median type		None			None							
Median storage veh												
Upstream signal (ft)		1185										
pX, platoon unblocked					0.81			0.81	0.81	0.81	0.81	0.81
vC, conflicting volume	1060				1011			2084	2083	1011	2086	2083
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	1060				897			2220	2219	897	2223	2219
tC, single (s)	4.1				4.1			7.1	6.5	6.5	7.1	6.5
tC, 2 stage (s)												
tF (s)	2.2				2.2			3.5	4.0	3.6	3.5	4.0
p0 queue free %	100				99			100	100	99	100	100
cM capacity (veh/h)	665				620			26	36	242	25	36
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	1011	1066	3	0								
Volume Left	0	6	0	0								
Volume Right	0	0	3	0								
cSH	665	620	242	1700								
Volume to Capacity	0.00	0.01	0.01	0.00								
Queue Length 95th (ft)	0	1	1	0								
Control Delay (s)	0.0	0.3	20.1	0.0								
Lane LOS		A	C	A								
Approach Delay (s)	0.0	0.3	20.1	0.0								
Approach LOS		C	A									
Intersection Summary												
Average Delay			0.2									
Intersection Capacity Utilization		64.5%			ICU Level of Service				C			
Analysis Period (min)			15									

Queues

3: SW Roy Rogers Rd/SW 175th Ave & SW Scholls Ferry Rd

01/18/2017

Lane Group	EBL	EBT	EBC	WBL	WBT	NBL	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	38	540	481	503	655	468	550	710	114	625
v/c Ratio	0.43	0.85	0.61	0.92	0.57	0.96	0.76	0.73	0.78	0.91
Control Delay	71.9	60.6	19.0	72.9	34.9	73.3	39.3	19.9	87.6	65.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	71.9	60.6	19.0	72.9	34.9	73.3	39.3	19.9	87.6	65.4
Queue Length 50th (ft)	30	218	198	204	225	365	375	338	89	254
Queue Length 95th (ft)	68	284	301	#309	293	#588	531	517	#180	#367
Internal Link Dist (ft)		686			809		447			409
Turn Bay Length (ft)	170		330	330		300		300	250	
Base Capacity (vph)	103	722	811	572	1156	508	743	985	168	718
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.37	0.75	0.59	0.88	0.57	0.92	0.74	0.72	0.68	0.87

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis
 3: SW Roy Rogers Rd/SW 175th Ave & SW Scholls Ferry Rd

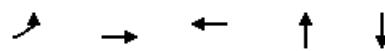
01/18/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑	↑	↑↑	↑↑		↑	↑	↑	↑	↑↑	
Traffic Volume (vph)	34	486	433	453	475	114	421	495	639	103	512	50
Future Volume (vph)	34	486	433	453	475	114	421	495	639	103	512	50
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)	0%			0%				3%			-2%	
Total Lost time (s)	4.0	5.5	4.0	4.0	5.5		4.0	5.5	4.0	4.0	5.5	
Lane Util. Factor	1.00	0.95	1.00	0.97	0.95		1.00	1.00	1.00	1.00	0.95	
Frbp, ped/bikes	1.00	1.00	0.99	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Fr _t	1.00	1.00	0.85	1.00	0.97		1.00	1.00	0.85	1.00	0.99	
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1736	3406	1516	3367	3409		1662	1782	1530	1770	3494	
Flt Permitted	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (perm)	1736	3406	1516	3367	3409		1662	1782	1530	1770	3494	
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	38	540	481	503	528	127	468	550	710	114	569	56
RTOR Reduction (vph)	0	0	47	0	16	0	0	0	37	0	6	0
Lane Group Flow (vph)	38	540	434	503	639	0	468	550	673	114	619	0
Confl. Peds. (#/hr)							1				1	
Confl. Bikes (#/hr)			2									
Heavy Vehicles (%)	4%	6%	6%	4%	3%	2%	7%	5%	4%	3%	3%	1%
Turn Type	Prot	NA	pm+ov	Prot	NA		Prot	NA	pm+ov	Prot	NA	
Protected Phases	5	2	3	1	6		3	8	1	7	4	
Permitted Phases			2					8				
Actuated Green, G (s)	3.9	23.9	58.8	19.2	39.2		34.9	48.2	67.4	9.8	23.1	
Effective Green, g (s)	3.9	23.9	58.8	19.2	39.2		34.9	48.2	67.4	9.8	23.1	
Actuated g/C Ratio	0.03	0.20	0.49	0.16	0.33		0.29	0.40	0.56	0.08	0.19	
Clearance Time (s)	4.0	5.5	4.0	4.0	5.5		4.0	5.5	4.0	4.0	5.5	
Vehicle Extension (s)	1.0	2.0	1.0	1.0	2.0		1.0	2.0	1.0	1.0	2.0	
Lane Grp Cap (vph)	56	677	742	538	1112		482	715	858	144	672	
v/s Ratio Prot	0.02	c0.16	0.17	c0.15	0.19		c0.28	0.31	0.13	0.06	c0.18	
v/s Ratio Perm			0.12					0.31				
v/c Ratio	0.68	0.80	0.58	0.93	0.57		0.97	0.77	0.78	0.79	0.92	
Uniform Delay, d1	57.5	45.8	21.9	49.8	33.5		42.1	31.1	20.7	54.1	47.6	
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	22.6	6.1	0.8	23.4	0.4		33.3	4.5	4.4	23.5	17.9	
Delay (s)	80.1	51.9	22.7	73.2	34.0		75.4	35.6	25.0	77.7	65.5	
Level of Service	F	D	C	E	C		E	D	C	E	E	
Approach Delay (s)			39.6		51.0			42.1			67.4	
Approach LOS			D		D			D			E	
Intersection Summary												
HCM 2000 Control Delay			47.7			HCM 2000 Level of Service			D			
HCM 2000 Volume to Capacity ratio			0.91									
Actuated Cycle Length (s)			120.1			Sum of lost time (s)			19.0			
Intersection Capacity Utilization			81.4%			ICU Level of Service			D			
Analysis Period (min)			15									
c Critical Lane Group												

Queues

1: Parking Lot/SW Tile Flat Rd & SW Scholls Ferry Rd

01/19/2017



Lane Group	EBL	EBT	WBT	NBT	SBT
Lane Group Flow (vph)	5	678	917	1	403
v/c Ratio	0.06	0.57	0.81	0.00	0.84
Control Delay	54.0	12.5	22.0	0.0	48.5
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	54.0	12.5	22.0	0.0	48.5
Queue Length 50th (ft)	3	218	373	0	203
Queue Length 95th (ft)	18	385	#912	0	#454
Internal Link Dist (ft)		230	1105	30	390
Turn Bay Length (ft)		130			
Base Capacity (vph)	83	1344	1155	441	479
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.06	0.50	0.79	0.00	0.84

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis
1: Parking Lot/SW Tile Flat Rd & SW Scholls Ferry Rd

01/19/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑			↔		↔	↔			↔	
Traffic Volume (vph)	5	678	0	1	555	361	0	0	1	395	0	8
Future Volume (vph)	5	678	0	1	555	361	0	0	1	395	0	8
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	12	12	12	12	12	12	11	11	11	12	12	12
Grade (%)		4%			2%			0%			0%	
Total Lost time (s)	4.0	5.5			5.5			4.0			4.5	
Lane Util. Factor	1.00	1.00			1.00			1.00			1.00	
Fr _t	1.00	1.00			0.95			0.86			1.00	
Flt Protected	0.95	1.00			1.00			1.00			0.95	
Satd. Flow (prot)	1769	1808			1736			1589			1772	
Flt Permitted	0.95	1.00			1.00			1.00			0.95	
Satd. Flow (perm)	1769	1808			1735			1589			1772	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	5	678	0	1	555	361	0	0	1	395	0	8
RTOR Reduction (vph)	0	0	0	0	14	0	0	1	0	0	65	0
Lane Group Flow (vph)	5	678	0	0	903	0	0	0	0	0	338	0
Heavy Vehicles (%)	0%	3%	0%	0%	3%	2%	0%	0%	0%	2%	0%	0%
Turn Type	Prot	NA		Perm	NA				NA		Split	NA
Protected Phases	5	2			6			8	8		4	4
Permitted Phases					6							
Actuated Green, G (s)	0.9	73.8			68.9			0.7			25.0	
Effective Green, g (s)	0.9	73.8			68.9			0.7			25.0	
Actuated g/C Ratio	0.01	0.65			0.61			0.01			0.22	
Clearance Time (s)	4.0	5.5			5.5			4.0			4.5	
Vehicle Extension (s)	1.5	3.5			3.5			1.5			1.5	
Lane Grp Cap (vph)	14	1175			1053			9			390	
v/s Ratio Prot	0.00	c0.38						c0.00			c0.19	
v/s Ratio Perm					c0.52							
v/c Ratio	0.36	0.58			0.86			0.00			0.87	
Uniform Delay, d1	56.0	11.1			18.3			56.1			42.6	
Progression Factor	1.00	1.00			1.00			1.00			1.00	
Incremental Delay, d2	5.6	0.7			7.2			0.0			17.3	
Delay (s)	61.6	11.9			25.5			56.1			59.9	
Level of Service	E	B			C			E			E	
Approach Delay (s)		12.2			25.5			56.1			59.9	
Approach LOS		B			C			E			E	
Intersection Summary												
HCM 2000 Control Delay		27.9			HCM 2000 Level of Service			C				
HCM 2000 Volume to Capacity ratio		0.86										
Actuated Cycle Length (s)		113.5			Sum of lost time (s)			18.0				
Intersection Capacity Utilization		89.4%			ICU Level of Service			E				
Analysis Period (min)		15										
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis
 2: SW Vandermost Rd/SW Strobel Rd & SW Scholls Ferry Rd

01/19/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	1	1059		3	3	912	1	2	0	14	1	1
Future Volume (Veh/h)	1	1059		3	3	912	1	2	0	14	1	1
Sign Control		Free				Free			Stop		Stop	
Grade		-4%				0%			-7%		-4%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	1	1059		3	3	912	1	2	0	14	1	1
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None				None						
Median storage veh												
Upstream signal (ft)		1185										
pX, platoon unblocked					0.77			0.77	0.77	0.77	0.77	0.77
vC, conflicting volume	913				1062			1982	1982	1060	1995	1982
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	913				929			2129	2127	927	2145	2129
tC, single (s)	4.1				4.1			7.1	6.5	6.2	7.1	6.5
tC, 2 stage (s)												
tF (s)	2.2				2.2			3.5	4.0	3.3	3.5	4.0
p0 queue free %	100				99			93	100	94	96	97
cM capacity (veh/h)	755				571			28	39	252	26	39
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	1063	916	16	3								
Volume Left	1	3	2	1								
Volume Right	3	1	14	1								
cSH	755	571	125	44								
Volume to Capacity	0.00	0.01	0.13	0.07								
Queue Length 95th (ft)	0	0	11	5								
Control Delay (s)	0.0	0.2	38.0	92.0								
Lane LOS	A	A	E	F								
Approach Delay (s)	0.0	0.2	38.0	92.0								
Approach LOS			E	F								
Intersection Summary												
Average Delay			0.5									
Intersection Capacity Utilization		66.6%			ICU Level of Service				C			
Analysis Period (min)			15									

Queues

3: SW Roy Rogers Rd/SW 175th Ave & SW Scholls Ferry Rd

01/19/2017



Lane Group	EBL	EBT	EBC	WBL	WBT	NBL	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	66	580	483	735	596	424	538	587	163	623
v/c Ratio	0.58	0.87	0.60	0.98	0.48	0.98	0.86	0.60	0.92	0.91
Control Delay	75.4	62.0	21.6	77.0	31.4	84.4	52.6	16.8	103.3	66.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	75.4	62.0	21.6	77.0	31.4	84.4	52.6	16.8	103.3	66.5
Queue Length 50th (ft)	52	235	219	~307	187	~340	399	253	131	255
Queue Length 95th (ft)	100	303	331	#442	251	#559	#600	373	#265	#361
Internal Link Dist (ft)		686			809		447			409
Turn Bay Length (ft)	170		330	330		300		300	250	
Base Capacity (vph)	149	741	810	747	1235	433	638	981	182	714
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.44	0.78	0.60	0.98	0.48	0.98	0.84	0.60	0.90	0.87

Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.
- Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
- Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis
 3: SW Roy Rogers Rd/SW 175th Ave & SW Scholls Ferry Rd

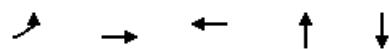
01/19/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑	↑	↑↑	↑↑		↑	↑	↑	↑	↑↑	
Traffic Volume (vph)	66	580	483	735	472	124	424	538	587	163	573	50
Future Volume (vph)	66	580	483	735	472	124	424	538	587	163	573	50
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)		0%			0%			3%			-2%	
Total Lost time (s)	4.0	5.5	4.0	4.0	5.5		4.0	5.5	4.0	4.0	5.5	
Lane Util. Factor	1.00	0.95	1.00	0.97	0.95		1.00	1.00	1.00	1.00	0.95	
Fr _t	1.00	1.00	0.85	1.00	0.97		1.00	1.00	0.85	1.00	0.99	
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1787	3574	1583	3467	3429		1743	1835	1591	1823	3564	
Flt Permitted	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (perm)	1787	3574	1583	3467	3429		1743	1835	1591	1823	3564	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	66	580	483	735	472	124	424	538	587	163	573	50
RTOR Reduction (vph)	0	0	51	0	18	0	0	0	26	0	5	0
Lane Group Flow (vph)	66	580	432	735	578	0	424	538	562	163	618	0
Heavy Vehicles (%)	1%	1%	2%	1%	2%	2%	2%	2%	0%	0%	1%	2%
Turn Type	Prot	NA	pm+ov	Prot	NA		Prot	NA	pm+ov	Prot	NA	
Protected Phases	5	2	3	1	6		3	8	1	7	4	
Permitted Phases			2					8				
Actuated Green, G (s)	6.7	23.6	53.6	26.0	42.9		30.0	41.2	67.2	11.8	23.0	
Effective Green, g (s)	6.7	23.6	53.6	26.0	42.9		30.0	41.2	67.2	11.8	23.0	
Actuated g/C Ratio	0.06	0.19	0.44	0.21	0.35		0.25	0.34	0.55	0.10	0.19	
Clearance Time (s)	4.0	5.5	4.0	4.0	5.5		4.0	5.5	4.0	4.0	5.5	
Vehicle Extension (s)	1.0	2.0	1.0	1.0	2.0		1.0	2.0	1.0	1.0	2.0	
Lane Grp Cap (vph)	98	693	697	741	1209		430	621	879	176	674	
v/s Ratio Prot	0.04	c0.16	0.15	c0.21	0.17		c0.24	c0.29	0.14	0.09	0.17	
v/s Ratio Perm			0.12					0.22				
v/c Ratio	0.67	0.84	0.62	0.99	0.48		0.99	0.87	0.64	0.93	0.92	
Uniform Delay, d1	56.4	47.1	26.1	47.7	30.6		45.6	37.6	18.8	54.5	48.4	
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	13.4	8.3	1.2	30.9	0.1		39.3	11.8	1.1	46.0	17.0	
Delay (s)	69.8	55.5	27.3	78.6	30.7		84.9	49.4	19.9	100.5	65.4	
Level of Service	E	E	C	E	C		F	D	B	F	E	
Approach Delay (s)		44.3			57.1			47.9			72.7	
Approach LOS		D			E			D			E	
Intersection Summary												
HCM 2000 Control Delay		53.7										
HCM 2000 Volume to Capacity ratio		0.95										
Actuated Cycle Length (s)		121.6										
Intersection Capacity Utilization		93.8%										
Analysis Period (min)		15										
c Critical Lane Group												

Queues

1: Parking Lot/SW Tile Flat Rd & SW Scholls Ferry Rd

01/18/2017



Lane Group	EBL	EBT	WBT	NBT	SBT
Lane Group Flow (vph)	5	737	996	1	438
v/c Ratio	0.06	0.61	0.87	0.00	0.94
Control Delay	54.2	13.1	25.7	0.0	63.5
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	54.2	13.1	25.7	0.0	63.5
Queue Length 50th (ft)	3	246	444	0	239
Queue Length 95th (ft)	18	437	#1042	0	#523
Internal Link Dist (ft)		230	1105	30	390
Turn Bay Length (ft)		130			
Base Capacity (vph)	82	1330	1145	417	466
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.06	0.55	0.87	0.00	0.94

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis
1: Parking Lot/SW Tile Flat Rd & SW Scholls Ferry Rd

01/18/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑			↔		↔	↔			↔	
Traffic Volume (vph)	5	678	0	1	555	361	0	0	1	395	0	8
Future Volume (vph)	5	678	0	1	555	361	0	0	1	395	0	8
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	12	12	12	12	12	12	11	11	11	12	12	12
Grade (%)		4%			2%			0%			0%	
Total Lost time (s)	4.0	5.5			5.5			4.0			4.5	
Lane Util. Factor	1.00	1.00			1.00			1.00			1.00	
Fr _t	1.00	1.00			0.95			0.86			1.00	
Flt Protected	0.95	1.00			1.00			1.00			0.95	
Satd. Flow (prot)	1769	1808			1736			1589			1772	
Flt Permitted	0.95	1.00			1.00			1.00			0.95	
Satd. Flow (perm)	1769	1808			1735			1589			1772	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	5	737	0	1	603	392	0	0	1	429	0	9
RTOR Reduction (vph)	0	0	0	0	13	0	0	1	0	0	66	0
Lane Group Flow (vph)	5	737	0	0	983	0	0	0	0	0	372	0
Heavy Vehicles (%)	0%	3%	0%	0%	3%	2%	0%	0%	0%	2%	0%	0%
Turn Type	Prot	NA		Perm	NA				NA		Split	NA
Protected Phases	5	2			6			8	8		4	4
Permitted Phases					6							
Actuated Green, G (s)	0.9	75.7			70.8			0.7			24.6	
Effective Green, g (s)	0.9	75.7			70.8			0.7			24.6	
Actuated g/C Ratio	0.01	0.66			0.62			0.01			0.21	
Clearance Time (s)	4.0	5.5			5.5			4.0			4.5	
Vehicle Extension (s)	1.5	3.5			3.5			1.5			1.5	
Lane Grp Cap (vph)	13	1190			1068			9			379	
v/s Ratio Prot	0.00	c0.41						c0.00			c0.21	
v/s Ratio Perm					c0.57							
v/c Ratio	0.38	0.62			0.92			0.00			0.98	
Uniform Delay, d1	56.8	11.3			19.6			56.8			45.0	
Progression Factor	1.00	1.00			1.00			1.00			1.00	
Incremental Delay, d2	6.8	1.0			12.6			0.0			41.0	
Delay (s)	63.5	12.4			32.1			56.8			86.0	
Level of Service	E	B			C			E			F	
Approach Delay (s)		12.7			32.1			56.8			86.0	
Approach LOS		B			C			E			F	
Intersection Summary												
HCM 2000 Control Delay		36.4			HCM 2000 Level of Service			D				
HCM 2000 Volume to Capacity ratio		0.94										
Actuated Cycle Length (s)		115.0			Sum of lost time (s)			18.0				
Intersection Capacity Utilization		89.4%			ICU Level of Service			E				
Analysis Period (min)		15										
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis
 2: SW Vandermost Rd/SW Strobel Rd & SW Scholls Ferry Rd

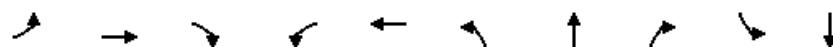
01/18/2017

Movement	EBL	EBT	EBC	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	1	1059		3	3	912	1	2	0	14	1	1
Future Volume (Veh/h)	1	1059		3	3	912	1	2	0	14	1	1
Sign Control		Free				Free			Stop		Stop	
Grade		-4%				0%			-7%		-4%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	1	1151		3	3	991	1	2	0	15	1	1
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None				None						
Median storage veh												
Upstream signal (ft)		1185										
pX, platoon unblocked					0.73			0.73	0.73	0.73	0.73	0.73
vC, conflicting volume	992				1154			2154	2152	1152	2167	2154
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	992				1026			2395	2393	1024	2413	2395
tC, single (s)	4.1				4.1			7.1	6.5	6.2	7.1	6.5
tC, 2 stage (s)												
tF (s)	2.2				2.2			3.5	4.0	3.3	3.5	4.0
p0 queue free %	100				99			88	100	93	94	96
cM capacity (veh/h)	705				500			17	25	211	16	25
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	1155	995	17	3								
Volume Left	1	3	2	1								
Volume Right	3	1	15	1								
cSH	705	500	89	28								
Volume to Capacity	0.00	0.01	0.19	0.11								
Queue Length 95th (ft)	0	0	16	8								
Control Delay (s)	0.1	0.2	54.7	149.7								
Lane LOS	A	A	F	F								
Approach Delay (s)	0.1	0.2	54.7	149.7								
Approach LOS			F	F								
Intersection Summary												
Average Delay			0.8									
Intersection Capacity Utilization		66.6%			ICU Level of Service				C			
Analysis Period (min)			15									

Queues

3: SW Roy Rogers Rd/SW 175th Ave & SW Scholls Ferry Rd

01/18/2017



Lane Group	EBL	EBT	EBC	WBL	WBT	NBL	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	69	604	503	766	621	442	560	611	170	649
v/c Ratio	0.59	0.89	0.62	1.04	0.50	1.03	0.90	0.63	0.92	0.94
Control Delay	76.3	63.8	22.5	89.3	32.0	96.8	58.5	18.1	103.7	70.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	76.3	63.8	22.5	89.3	32.0	96.8	58.5	18.1	103.7	70.7
Queue Length 50th (ft)	55	247	235	~345	198	~385	429	278	137	270
Queue Length 95th (ft)	104	#333	353	#469	264	#591	#645	403	#274	#385
Internal Link Dist (ft)		686			809		447			409
Turn Bay Length (ft)	170		330	330		300		300	250	
Base Capacity (vph)	151	734	811	740	1236	429	624	972	188	707
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.46	0.82	0.62	1.04	0.50	1.03	0.90	0.63	0.90	0.92

Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.
- Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
- Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis
 3: SW Roy Rogers Rd/SW 175th Ave & SW Scholls Ferry Rd

01/18/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑	↑	↑↑	↑↑		↑	↑	↑	↑	↑↑	
Traffic Volume (vph)	66	580	483	735	472	124	424	538	587	163	573	50
Future Volume (vph)	66	580	483	735	472	124	424	538	587	163	573	50
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)		0%			0%			3%			-2%	
Total Lost time (s)	4.0	5.5	4.0	4.0	5.5		4.0	5.5	4.0	4.0	5.5	
Lane Util. Factor	1.00	0.95	1.00	0.97	0.95		1.00	1.00	1.00	1.00	0.95	
Fr _t	1.00	1.00	0.85	1.00	0.97		1.00	1.00	0.85	1.00	0.99	
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1787	3574	1583	3467	3429		1743	1835	1591	1823	3564	
Flt Permitted	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (perm)	1787	3574	1583	3467	3429		1743	1835	1591	1823	3564	
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	69	604	503	766	492	129	442	560	611	170	597	52
RTOR Reduction (vph)	0	0	51	0	18	0	0	0	26	0	5	0
Lane Group Flow (vph)	69	604	452	766	603	0	442	560	585	170	644	0
Heavy Vehicles (%)	1%	1%	2%	1%	2%	2%	2%	2%	0%	0%	1%	2%
Turn Type	Prot	NA	pm+ov	Prot	NA		Prot	NA	pm+ov	Prot	NA	
Protected Phases	5	2	3	1	6		3	8	1	7	4	
Permitted Phases			2						8			
Actuated Green, G (s)	6.9	24.2	54.2	26.0	43.3		30.0	41.1	67.1	12.3	23.4	
Effective Green, g (s)	6.9	24.2	54.2	26.0	43.3		30.0	41.1	67.1	12.3	23.4	
Actuated g/C Ratio	0.06	0.20	0.44	0.21	0.35		0.24	0.34	0.55	0.10	0.19	
Clearance Time (s)	4.0	5.5	4.0	4.0	5.5		4.0	5.5	4.0	4.0	5.5	
Vehicle Extension (s)	1.0	2.0	1.0	1.0	2.0		1.0	2.0	1.0	1.0	2.0	
Lane Grp Cap (vph)	100	705	699	735	1211		426	615	870	182	680	
v/s Ratio Prot	0.04	c0.17	0.16	c0.22	0.18		c0.25	c0.31	0.14	0.09	0.18	
v/s Ratio Perm			0.13						0.23			
v/c Ratio	0.69	0.86	0.65	1.04	0.50		1.04	0.91	0.67	0.93	0.95	
Uniform Delay, d1	56.8	47.5	26.7	48.3	31.1		46.3	39.0	19.9	54.7	49.0	
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	14.6	9.7	1.5	44.7	0.1		53.7	17.4	1.6	47.2	22.0	
Delay (s)	71.4	57.2	28.3	93.0	31.2		100.0	56.4	21.5	102.0	71.0	
Level of Service	E	E	C	F	C		F	E	C	F	E	
Approach Delay (s)		45.7			65.3			55.1			77.4	
Approach LOS		D			E			E			E	
Intersection Summary												
HCM 2000 Control Delay		59.4										E
HCM 2000 Volume to Capacity ratio		0.99										
Actuated Cycle Length (s)		122.6										19.0
Intersection Capacity Utilization		93.8%										F
Analysis Period (min)		15										
c Critical Lane Group												

Appendix E Year 2018 Total Traffic Conditions

Queues

1: Parking Lot/SW Tile Flat Rd & SW Scholls Ferry Rd

01/19/2017



Lane Group	EBL	EBT	WBT	SBT
Lane Group Flow (vph)	4	608	969	328
v/c Ratio	0.05	0.50	0.82	0.82
Control Delay	51.8	8.7	19.6	47.9
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	51.8	8.7	19.6	47.9
Queue Length 50th (ft)	3	165	380	159
Queue Length 95th (ft)	14	235	#876	#336
Internal Link Dist (ft)		230	1105	390
Turn Bay Length (ft)	130			
Base Capacity (vph)	83	1338	1175	405
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.05	0.45	0.82	0.81

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis
1: Parking Lot/SW Tile Flat Rd & SW Scholls Ferry Rd

01/19/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑			↔		↔	↔			↔	
Traffic Volume (vph)	4	608	0	0	541	428	0	0	0	319	0	9
Future Volume (vph)	4	608	0	0	541	428	0	0	0	319	0	9
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	12	12	12	12	12	12	11	11	11	12	12	12
Grade (%)		4%			2%				0%		0%	
Total Lost time (s)	4.0	5.5			5.5					4.5		
Lane Util. Factor	1.00	1.00			1.00					1.00		
Fr _t	1.00	1.00			0.94					1.00		
Flt Protected	0.95	1.00			1.00					0.95		
Satd. Flow (prot)	1769	1724			1679					1691		
Flt Permitted	0.95	1.00			1.00					0.95		
Satd. Flow (perm)	1769	1724			1679					1691		
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	4	608	0	0	541	428	0	0	0	319	0	9
RTOR Reduction (vph)	0	0	0	0	15	0	0	0	0	0	68	0
Lane Group Flow (vph)	4	608	0	0	954	0	0	0	0	0	260	0
Heavy Vehicles (%)	0%	8%	0%	0%	8%	2%	0%	0%	0%	6%	0%	33%
Turn Type	Prot	NA			NA					Split	NA	
Protected Phases	5	2			6		8	8		4	4	
Permitted Phases					6							
Actuated Green, G (s)	0.9	78.7			73.8					21.1		
Effective Green, g (s)	0.9	78.7			73.8					21.1		
Actuated g/C Ratio	0.01	0.72			0.67					0.19		
Clearance Time (s)	4.0	5.5			5.5					4.5		
Vehicle Extension (s)	1.5	3.5			3.5					1.5		
Lane Grp Cap (vph)	14	1235			1128					324		
v/s Ratio Prot	0.00	c0.35			c0.57					c0.15		
v/s Ratio Perm												
v/c Ratio	0.29	0.49			0.85					0.80		
Uniform Delay, d1	54.1	6.8			13.7					42.4		
Progression Factor	1.00	1.00			1.00					1.00		
Incremental Delay, d2	4.1	0.4			6.1					12.7		
Delay (s)	58.2	7.2			19.8					55.0		
Level of Service	E	A			B					E		
Approach Delay (s)		7.5			19.8		0.0			55.0		
Approach LOS		A			B		A			E		
Intersection Summary												
HCM 2000 Control Delay		21.9			HCM 2000 Level of Service					C		
HCM 2000 Volume to Capacity ratio		0.88										
Actuated Cycle Length (s)		109.8			Sum of lost time (s)					18.0		
Intersection Capacity Utilization		81.2%			ICU Level of Service					D		
Analysis Period (min)		15										
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis
 2: SW Vandermost Rd/SW Strobel Rd & SW Scholls Ferry Rd

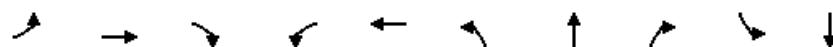
01/19/2017

Movement	EBL	EBT	EBC	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↑	↑	↑	↑	↔		↑	↑	
Traffic Volume (veh/h)	6	910	0	5	954	35	0	0	3	122	0	22
Future Volume (Veh/h)	6	910	0	5	954	35	0	0	3	122	0	22
Sign Control	Free			Free			Stop			Stop		
Grade	-4%			0%			-7%			-4%		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	6	910	0	5	954	35	0	0	3	122	0	22
Pedestrians	1											
Lane Width (ft)	12.0											
Walking Speed (ft/s)	3.5											
Percent Blockage	0											
Right turn flare (veh)												
Median type	None			TWLTL								
Median storage veh				2								
Upstream signal (ft)	1185											
pX, platoon unblocked				0.85			0.85	0.85	0.85	0.85	0.85	
vC, conflicting volume	989			910			1909	1921	910	1889	1886	955
vC1, stage 1 conf vol							922	922		964	964	
vC2, stage 2 conf vol							987	999		925	922	
vCu, unblocked vol	989			809			1979	1993	809	1956	1952	955
tC, single (s)	4.1			4.1			7.1	6.5	6.5	7.1	6.5	6.2
tC, 2 stage (s)							6.1	5.5		6.1	5.5	
tF (s)	2.2			2.2			3.5	4.0	3.6	3.5	4.0	3.3
p0 queue free %	99			99			100	100	99	43	100	93
cM capacity (veh/h)	707			705			202	228	288	213	233	316
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	WB 3	NB 1	SB 1	SB 2				
Volume Total	6	910	5	954	35	3	122	22				
Volume Left	6	0	5	0	0	0	122	0				
Volume Right	0	0	0	0	35	3	0	22				
cSH	707	1700	705	1700	1700	288	213	316				
Volume to Capacity	0.01	0.54	0.01	0.56	0.02	0.01	0.57	0.07				
Queue Length 95th (ft)	1	0	1	0	0	1	79	6				
Control Delay (s)	10.1	0.0	10.1	0.0	0.0	17.6	42.5	17.2				
Lane LOS	B		B			C	E	C				
Approach Delay (s)	0.1		0.1			17.6	38.7					
Approach LOS						C	E					
Intersection Summary												
Average Delay				2.8								
Intersection Capacity Utilization				70.3%			ICU Level of Service			C		
Analysis Period (min)				15								

Queues

3: SW Roy Rogers Rd/SW 175th Ave & SW Scholls Ferry Rd

01/19/2017



Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	48	580	447	453	616	425	495	639	103	566
v/c Ratio	0.46	0.84	0.56	0.87	0.56	0.92	0.72	0.68	0.72	0.84
Control Delay	69.2	55.7	16.8	65.8	34.7	67.1	37.1	18.3	80.8	57.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	69.2	55.7	16.8	65.8	34.7	67.1	37.1	18.3	80.8	57.1
Queue Length 50th (ft)	37	232	171	181	208	321	322	278	80	223
Queue Length 95th (ft)	79	300	263	#275	276	#521	464	434	#163	#311
Internal Link Dist (ft)		686			809		447			409
Turn Bay Length (ft)	170		330	330		300		300	250	
Base Capacity (vph)	143	833	852	579	1161	527	780	966	171	765
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.34	0.70	0.52	0.78	0.53	0.81	0.63	0.66	0.60	0.74

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis
3: SW Roy Rogers Rd/SW 175th Ave & SW Scholls Ferry Rd

01/19/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑	↑	↑↑	↑↑		↑	↑	↑	↑	↑↑	
Traffic Volume (vph)	48	580	447	453	502	114	425	495	639	103	512	54
Future Volume (vph)	48	580	447	453	502	114	425	495	639	103	512	54
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)	0%			0%				3%			-2%	
Total Lost time (s)	4.0	5.5	4.0	4.0	5.5		4.0	5.5	4.0	4.0	5.5	
Lane Util. Factor	1.00	0.95	1.00	0.97	0.95		1.00	1.00	1.00	1.00	0.95	
Frbp, ped/bikes	1.00	1.00	0.99	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Fr _t	1.00	1.00	0.85	1.00	0.97		1.00	1.00	0.85	1.00	0.99	
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1736	3406	1515	3367	3414		1662	1782	1530	1770	3491	
Flt Permitted	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (perm)	1736	3406	1515	3367	3414		1662	1782	1530	1770	3491	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	48	580	447	453	502	114	425	495	639	103	512	54
RTOR Reduction (vph)	0	0	47	0	15	0	0	0	40	0	6	0
Lane Group Flow (vph)	48	580	400	453	601	0	425	495	599	103	560	0
Confl. Peds. (#/hr)							1				1	
Confl. Bikes (#/hr)			2									
Heavy Vehicles (%)	4%	6%	6%	4%	3%	2%	7%	5%	4%	3%	3%	1%
Turn Type	Prot	NA	pm+ov	Prot	NA		Prot	NA	pm+ov	Prot	NA	
Protected Phases	5	2	3	1	6		3	8	1	7	4	
Permitted Phases			2					8				
Actuated Green, G (s)	5.5	24.0	55.4	17.5	36.0		31.4	43.8	61.3	9.1	21.5	
Effective Green, g (s)	5.5	24.0	55.4	17.5	36.0		31.4	43.8	61.3	9.1	21.5	
Actuated g/C Ratio	0.05	0.21	0.49	0.15	0.32		0.28	0.39	0.54	0.08	0.19	
Clearance Time (s)	4.0	5.5	4.0	4.0	5.5		4.0	5.5	4.0	4.0	5.5	
Vehicle Extension (s)	1.0	2.0	1.0	1.0	2.0		1.0	2.0	1.0	1.0	2.0	
Lane Grp Cap (vph)	84	720	740	519	1083		460	688	827	142	661	
v/s Ratio Prot	0.03	c0.17	0.15	c0.13	0.18		c0.26	0.28	0.11	0.06	c0.16	
v/s Ratio Perm			0.11					0.28				
v/c Ratio	0.57	0.81	0.54	0.87	0.55		0.92	0.72	0.72	0.73	0.85	
Uniform Delay, d1	52.8	42.5	20.2	46.9	32.1		39.8	29.6	19.7	50.9	44.4	
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	5.7	6.2	0.4	14.5	0.4		23.9	3.0	2.7	14.4	9.4	
Delay (s)	58.5	48.7	20.6	61.4	32.4		63.8	32.6	22.4	65.3	53.8	
Level of Service	E	D	C	E	C		E	C	C	E	D	
Approach Delay (s)		37.4			44.7			36.9			55.5	
Approach LOS		D			D			D			E	
Intersection Summary												
HCM 2000 Control Delay		41.8										D
HCM 2000 Volume to Capacity ratio		0.87										
Actuated Cycle Length (s)		113.4										19.0
Intersection Capacity Utilization		84.4%										E
Analysis Period (min)		15										
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis

4: SW Strobel Rd & New Collector

01/19/2017

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↖ ↗ ↘ ↖ ↙ ↘					
Traffic Volume (veh/h)	48	0	28	13	0	96
Future Volume (Veh/h)	48	0	28	13	0	96
Sign Control	Stop		Free			Free
Grade	0%		0%			-4%
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	48	0	28	13	0	96
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None			None
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	130	34			41	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	130	34			41	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	94	100			100	
cM capacity (veh/h)	868	1044			1581	
Direction, Lane #	WB 1	WB 2	NB 1	SB 1		
Volume Total	48	0	41	96		
Volume Left	48	0	0	0		
Volume Right	0	0	13	0		
cSH	868	1700	1700	1581		
Volume to Capacity	0.06	0.00	0.02	0.00		
Queue Length 95th (ft)	4	0	0	0		
Control Delay (s)	9.4	0.0	0.0	0.0		
Lane LOS	A	A				
Approach Delay (s)	9.4		0.0	0.0		
Approach LOS	A					
Intersection Summary						
Average Delay		2.4				
Intersection Capacity Utilization		15.1%		ICU Level of Service		A
Analysis Period (min)		15				

Queues

1: Parking Lot/SW Tile Flat Rd & SW Scholls Ferry Rd

01/19/2017



Lane Group	EBL	EBT	WBT	SBT
Lane Group Flow (vph)	4	676	1077	364
v/c Ratio	0.05	0.56	0.92	0.89
Control Delay	51.8	9.7	28.4	57.4
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	51.8	9.7	28.4	57.4
Queue Length 50th (ft)	3	197	506	188
Queue Length 95th (ft)	14	280	#1051	#396
Internal Link Dist (ft)		230	1105	390
Turn Bay Length (ft)	130			
Base Capacity (vph)	83	1333	1170	408
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.05	0.51	0.92	0.89

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis
1: Parking Lot/SW Tile Flat Rd & SW Scholls Ferry Rd

01/19/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑			↔		↔	↔			↔	
Traffic Volume (vph)	4	608	0	0	541	428	0	0	0	319	0	9
Future Volume (vph)	4	608	0	0	541	428	0	0	0	319	0	9
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	12	12	12	12	12	12	11	11	11	12	12	12
Grade (%)		4%			2%				0%		0%	
Total Lost time (s)	4.0	5.5			5.5					4.5		
Lane Util. Factor	1.00	1.00			1.00					1.00		
Fr _t	1.00	1.00			0.94					1.00		
Flt Protected	0.95	1.00			1.00					0.95		
Satd. Flow (prot)	1769	1724			1679					1691		
Flt Permitted	0.95	1.00			1.00					0.95		
Satd. Flow (perm)	1769	1724			1679					1691		
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	4	676	0	0	601	476	0	0	0	354	0	10
RTOR Reduction (vph)	0	0	0	0	15	0	0	0	0	0	68	0
Lane Group Flow (vph)	4	676	0	0	1062	0	0	0	0	0	296	0
Heavy Vehicles (%)	0%	8%	0%	0%	8%	2%	0%	0%	0%	6%	0%	33%
Turn Type	Prot	NA			NA					Split	NA	
Protected Phases	5	2			6		8	8		4	4	
Permitted Phases					6							
Actuated Green, G (s)	0.9	78.5			73.6					21.5		
Effective Green, g (s)	0.9	78.5			73.6					21.5		
Actuated g/C Ratio	0.01	0.71			0.67					0.20		
Clearance Time (s)	4.0	5.5			5.5					4.5		
Vehicle Extension (s)	1.5	3.5			3.5					1.5		
Lane Grp Cap (vph)	14	1230			1123					330		
v/s Ratio Prot	0.00	c0.39			c0.63					c0.18		
v/s Ratio Perm												
v/c Ratio	0.29	0.55			0.95					0.90		
Uniform Delay, d1	54.2	7.4			16.4					43.2		
Progression Factor	1.00	1.00			1.00					1.00		
Incremental Delay, d2	4.1	0.6			15.6					25.0		
Delay (s)	58.3	8.0			32.0					68.2		
Level of Service	E	A			C					E		
Approach Delay (s)		8.3			32.0		0.0			68.2		
Approach LOS		A			C		A			E		
Intersection Summary												
HCM 2000 Control Delay		30.6			HCM 2000 Level of Service					C		
HCM 2000 Volume to Capacity ratio		0.98										
Actuated Cycle Length (s)		110.0			Sum of lost time (s)					18.0		
Intersection Capacity Utilization		81.2%			ICU Level of Service					D		
Analysis Period (min)		15										
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis
 2: SW Vandermost Rd/SW Strobel Rd & SW Scholls Ferry Rd

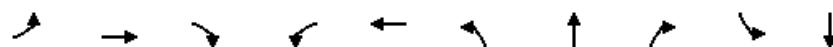
01/19/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖ ↗	↑ ↘		↖ ↗	↑ ↗	↖ ↗		↖ ↗		↖ ↗	↑ ↗	
Traffic Volume (veh/h)	6	910	0	5	954	35	0	0	3	122	0	22
Future Volume (Veh/h)	6	910	0	5	954	35	0	0	3	122	0	22
Sign Control	Free			Free			Stop			Stop		
Grade	-4%			0%			-7%			-4%		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	7	1011	0	6	1060	39	0	0	3	136	0	24
Pedestrians	1											
Lane Width (ft)	12.0											
Walking Speed (ft/s)	3.5											
Percent Blockage	0											
Right turn flare (veh)												
Median type	None			TWLTL								
Median storage veh					2							
Upstream signal (ft)	1185											
pX, platoon unblocked				0.81			0.81	0.81	0.81	0.81	0.81	
vC, conflicting volume	1099			1011			2122	2136	1011	2100	2097	1061
vC1, stage 1 conf vol							1025	1025		1072	1072	
vC2, stage 2 conf vol							1097	1111		1028	1025	
vCu, unblocked vol	1099			896			2268	2286	896	2241	2237	1061
tC, single (s)	4.1			4.1			7.1	6.5	6.5	7.1	6.5	6.2
tC, 2 stage (s)							6.1	5.5		6.1	5.5	
tF (s)	2.2			2.2			3.5	4.0	3.6	3.5	4.0	3.3
p0 queue free %	99			99			100	100	99	23	100	91
cM capacity (veh/h)	643			620			167	194	242	177	198	274
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	WB 3	NB 1	SB 1	SB 2				
Volume Total	7	1011	6	1060	39	3	136	24				
Volume Left	7	0	6	0	0	0	136	0				
Volume Right	0	0	0	0	39	3	0	24				
cSH	643	1700	620	1700	1700	242	177	274				
Volume to Capacity	0.01	0.59	0.01	0.62	0.02	0.01	0.77	0.09				
Queue Length 95th (ft)	1	0	1	0	0	1	125	7				
Control Delay (s)	10.7	0.0	10.9	0.0	0.0	20.1	71.5	19.4				
Lane LOS	B		B			C	F	C				
Approach Delay (s)	0.1		0.1			20.1	63.7					
Approach LOS						C	F					
Intersection Summary												
Average Delay			4.5									
Intersection Capacity Utilization		70.3%		ICU Level of Service					C			
Analysis Period (min)			15									

Queues

3: SW Roy Rogers Rd/SW 175th Ave & SW Scholls Ferry Rd

01/19/2017



Lane Group	EBL	EBT	EBC	WBL	WBT	NBL	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	53	644	497	503	685	472	550	710	114	629
v/c Ratio	0.52	0.94	0.61	0.94	0.62	0.97	0.77	0.74	0.79	0.93
Control Delay	74.4	70.6	19.3	78.2	38.3	78.3	40.7	21.5	90.4	69.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	74.4	70.6	19.3	78.2	38.3	78.3	40.7	21.5	90.4	69.7
Queue Length 50th (ft)	42	269	209	208	242	376	382	357	90	260
Queue Length 95th (ft)	85	#382	317	#312	318	#597	531	527	#180	#371
Internal Link Dist (ft)		686			809		447			409
Turn Bay Length (ft)	170		330	330		300		300	250	
Base Capacity (vph)	134	705	817	547	1097	491	718	962	162	695
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.40	0.91	0.61	0.92	0.62	0.96	0.77	0.74	0.70	0.91

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis
3: SW Roy Rogers Rd/SW 175th Ave & SW Scholls Ferry Rd

01/19/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑	↑	↑↑	↑↑		↑	↑	↑	↑	↑↑	
Traffic Volume (vph)	48	580	447	453	502	114	425	495	639	103	512	54
Future Volume (vph)	48	580	447	453	502	114	425	495	639	103	512	54
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)	0%			0%				3%			-2%	
Total Lost time (s)	4.0	5.5	4.0	4.0	5.5		4.0	5.5	4.0	4.0	5.5	
Lane Util. Factor	1.00	0.95	1.00	0.97	0.95		1.00	1.00	1.00	1.00	0.95	
Frbp, ped/bikes	1.00	1.00	0.99	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Fr _t	1.00	1.00	0.85	1.00	0.97		1.00	1.00	0.85	1.00	0.99	
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1736	3406	1515	3367	3414		1662	1782	1530	1770	3491	
Flt Permitted	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (perm)	1736	3406	1515	3367	3414		1662	1782	1530	1770	3491	
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	53	644	497	503	558	127	472	550	710	114	569	60
RTOR Reduction (vph)	0	0	46	0	15	0	0	0	33	0	6	0
Lane Group Flow (vph)	53	644	451	503	670	0	472	550	677	114	623	0
Confl. Peds. (#/hr)							1				1	
Confl. Bikes (#/hr)			2									
Heavy Vehicles (%)	4%	6%	6%	4%	3%	2%	7%	5%	4%	3%	3%	1%
Turn Type	Prot	NA	pm+ov	Prot	NA		Prot	NA	pm+ov	Prot	NA	
Protected Phases	5	2	3	1	6		3	8	1	7	4	
Permitted Phases			2						8			
Actuated Green, G (s)	6.1	25.4	61.0	19.3	38.6		35.6	49.0	68.3	9.9	23.3	
Effective Green, g (s)	6.1	25.4	61.0	19.3	38.6		35.6	49.0	68.3	9.9	23.3	
Actuated g/C Ratio	0.05	0.21	0.50	0.16	0.31		0.29	0.40	0.56	0.08	0.19	
Clearance Time (s)	4.0	5.5	4.0	4.0	5.5		4.0	5.5	4.0	4.0	5.5	
Vehicle Extension (s)	1.0	2.0	1.0	1.0	2.0		1.0	2.0	1.0	1.0	2.0	
Lane Grp Cap (vph)	86	705	753	530	1074		482	712	852	142	663	
v/s Ratio Prot	0.03	c0.19	0.17	c0.15	0.20		c0.28	0.31	0.13	0.06	c0.18	
v/s Ratio Perm			0.12						0.32			
v/c Ratio	0.62	0.91	0.60	0.95	0.62		0.98	0.77	0.79	0.80	0.94	
Uniform Delay, d1	57.1	47.5	22.0	51.2	35.8		43.1	32.0	21.6	55.4	48.9	
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	8.9	16.0	0.9	26.3	0.8		35.1	4.8	4.8	25.6	20.8	
Delay (s)	66.0	63.5	22.9	77.5	36.6		78.2	36.7	26.4	81.0	69.7	
Level of Service	E	E	C	E	D		E	D	C	F	E	
Approach Delay (s)			46.7		53.9				43.8		71.5	
Approach LOS			D		D				D		E	
Intersection Summary												
HCM 2000 Control Delay			51.2			HCM 2000 Level of Service			D			
HCM 2000 Volume to Capacity ratio			0.95									
Actuated Cycle Length (s)			122.6			Sum of lost time (s)			19.0			
Intersection Capacity Utilization			84.4%			ICU Level of Service			E			
Analysis Period (min)			15									
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis

4: SW Strobel Rd & New Collector

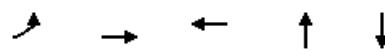
01/19/2017

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↖ ↗ ↘ ↖ ↙ ↘					
Traffic Volume (veh/h)	48	0	28	13	0	96
Future Volume (Veh/h)	48	0	28	13	0	96
Sign Control	Stop		Free			Free
Grade	0%		0%			-4%
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88
Hourly flow rate (vph)	55	0	32	15	0	109
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None			None
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	148	40		47		
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	148	40		47		
tC, single (s)	6.4	6.2		4.1		
tC, 2 stage (s)						
tF (s)	3.5	3.3		2.2		
p0 queue free %	94	100		100		
cM capacity (veh/h)	848	1038		1573		
Direction, Lane #	WB 1	WB 2	NB 1	SB 1		
Volume Total	55	0	47	109		
Volume Left	55	0	0	0		
Volume Right	0	0	15	0		
cSH	848	1700	1700	1573		
Volume to Capacity	0.06	0.00	0.03	0.00		
Queue Length 95th (ft)	5	0	0	0		
Control Delay (s)	9.5	0.0	0.0	0.0		
Lane LOS	A	A				
Approach Delay (s)	9.5		0.0	0.0		
Approach LOS	A					
Intersection Summary						
Average Delay		2.5				
Intersection Capacity Utilization		15.1%		ICU Level of Service		A
Analysis Period (min)		15				

Queues

1: Parking Lot/SW Tile Flat Rd & SW Scholls Ferry Rd

01/19/2017



Lane Group	EBL	EBT	WBT	NBT	SBT
Lane Group Flow (vph)	5	693	930	1	410
v/c Ratio	0.06	0.58	0.82	0.00	0.86
Control Delay	54.2	12.9	22.8	0.0	50.4
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	54.2	12.9	22.8	0.0	50.4
Queue Length 50th (ft)	3	229	390	0	213
Queue Length 95th (ft)	18	402	#940	0	#463
Internal Link Dist (ft)		230	1105	30	390
Turn Bay Length (ft)		130			
Base Capacity (vph)	82	1317	1132	431	479
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.06	0.53	0.82	0.00	0.86

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis
1: Parking Lot/SW Tile Flat Rd & SW Scholls Ferry Rd

01/19/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑			↔			↔			↔	
Traffic Volume (vph)	5	693	0	1	563	366	0	0	1	402	0	8
Future Volume (vph)	5	693	0	1	563	366	0	0	1	402	0	8
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	12	12	12	12	12	12	11	11	11	12	12	12
Grade (%)		4%			2%			0%			0%	
Total Lost time (s)	4.0	5.5			5.5			4.0			4.5	
Lane Util. Factor	1.00	1.00			1.00			1.00			1.00	
Fr _t	1.00	1.00			0.95			0.86			1.00	
Flt Protected	0.95	1.00			1.00			1.00			0.95	
Satd. Flow (prot)	1769	1808			1736			1589			1772	
Flt Permitted	0.95	1.00			1.00			1.00			0.95	
Satd. Flow (perm)	1769	1808			1735			1589			1772	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	5	693	0	1	563	366	0	0	1	402	0	8
RTOR Reduction (vph)	0	0	0	0	14	0	0	1	0	0	65	0
Lane Group Flow (vph)	5	693	0	0	916	0	0	0	0	0	345	0
Heavy Vehicles (%)	0%	3%	0%	0%	3%	2%	0%	0%	0%	2%	0%	0%
Turn Type	Prot	NA		Perm	NA			NA		Split	NA	
Protected Phases	5	2			6			8	8	4	4	
Permitted Phases					6							
Actuated Green, G (s)	0.9	74.9			70.0			0.7			25.4	
Effective Green, g (s)	0.9	74.9			70.0			0.7			25.4	
Actuated g/C Ratio	0.01	0.65			0.61			0.01			0.22	
Clearance Time (s)	4.0	5.5			5.5			4.0			4.5	
Vehicle Extension (s)	1.5	3.5			3.5			1.5			1.5	
Lane Grp Cap (vph)	13	1177			1056			9			391	
v/s Ratio Prot	0.00	c0.38						c0.00			c0.19	
v/s Ratio Perm					c0.53							
v/c Ratio	0.38	0.59			0.87			0.00			0.88	
Uniform Delay, d1	56.8	11.3			18.7			56.8			43.3	
Progression Factor	1.00	1.00			1.00			1.00			1.00	
Incremental Delay, d2	6.8	0.8			7.9			0.0			19.6	
Delay (s)	63.5	12.2			26.5			56.8			62.9	
Level of Service	E	B			C			E			E	
Approach Delay (s)		12.5			26.5			56.8			62.9	
Approach LOS		B			C			E			E	
Intersection Summary												
HCM 2000 Control Delay		29.1			HCM 2000 Level of Service			C				
HCM 2000 Volume to Capacity ratio		0.87										
Actuated Cycle Length (s)		115.0			Sum of lost time (s)			18.0				
Intersection Capacity Utilization		90.6%			ICU Level of Service			E				
Analysis Period (min)		15										
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis
 2: SW Vandermost Rd/SW Strobel Rd & SW Scholls Ferry Rd

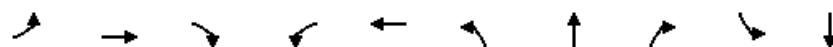
01/19/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↑	↑	↑	↑	↔		↑	↑	
Traffic Volume (veh/h)	23	1059	3	3	912	129	2	0	14	72	1	14
Future Volume (Veh/h)	23	1059	3	3	912	129	2	0	14	72	1	14
Sign Control	Free			Free			Stop			Stop		
Grade	-4%			0%			-7%			-4%		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	23	1059	3	3	912	129	2	0	14	72	1	14
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	None				TWLTL							
Median storage veh					2							
Upstream signal (ft)	1185											
pX, platoon unblocked				0.76			0.76	0.76	0.76	0.76	0.76	
vC, conflicting volume	1041			1062			2039	2154	1060	2037	2026	912
vC1, stage 1 conf vol						1106	1106			918	918	
vC2, stage 2 conf vol						932	1047			1119	1108	
vCu, unblocked vol	1041			927			2205	2355	925	2202	2188	912
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)						6.1	5.5			6.1	5.5	
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	97			99			99	100	94	57	99	96
cM capacity (veh/h)	676			570			169	179	252	166	193	335
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	WB 3	NB 1	SB 1	SB 2				
Volume Total	23	1062	3	912	129	16	72	15				
Volume Left	23	0	3	0	0	2	72	0				
Volume Right	0	3	0	0	129	14	0	14				
cSH	676	1700	570	1700	1700	237	166	319				
Volume to Capacity	0.03	0.62	0.01	0.54	0.08	0.07	0.43	0.05				
Queue Length 95th (ft)	3	0	0	0	0	5	49	4				
Control Delay (s)	10.5	0.0	11.3	0.0	0.0	21.3	42.2	16.8				
Lane LOS	B		B			C	E	C				
Approach Delay (s)	0.2		0.0			21.3	37.8					
Approach LOS						C	E					
Intersection Summary												
Average Delay				1.7								
Intersection Capacity Utilization			73.2%			ICU Level of Service			D			
Analysis Period (min)			15									

Queues

3: SW Roy Rogers Rd/SW 175th Ave & SW Scholls Ferry Rd

01/19/2017



Lane Group	EBL	EBT	EBC	WBL	WBT	NBL	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	74	635	491	735	694	439	538	587	163	638
v/c Ratio	0.61	0.93	0.64	1.00	0.57	1.01	0.85	0.60	0.93	0.93
Control Delay	75.8	69.3	23.1	82.0	34.5	91.1	52.0	16.9	106.8	70.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	75.8	69.3	23.1	82.0	34.5	91.1	52.0	16.9	106.8	70.0
Queue Length 50th (ft)	59	264	226	~311	234	~372	399	252	132	263
Queue Length 95th (ft)	108	#370	339	#442	310	#579	#592	367	#265	#374
Internal Link Dist (ft)		686			809		447			409
Turn Bay Length (ft)	170		330	330		300		300	250	
Base Capacity (vph)	169	708	766	734	1222	436	637	978	179	702
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.44	0.90	0.64	1.00	0.57	1.01	0.84	0.60	0.91	0.91

Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.
- Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
- Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis
 3: SW Roy Rogers Rd/SW 175th Ave & SW Scholls Ferry Rd

01/19/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑	↑	↑↑	↑↑		↑	↑	↑	↑	↑↑	
Traffic Volume (vph)	74	635	491	735	570	124	439	538	587	163	573	65
Future Volume (vph)	74	635	491	735	570	124	439	538	587	163	573	65
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)		0%			0%			3%			-2%	
Total Lost time (s)	4.0	5.5	4.0	4.0	5.5		4.0	5.5	4.0	4.0	5.5	
Lane Util. Factor	1.00	0.95	1.00	0.97	0.95		1.00	1.00	1.00	1.00	0.95	
Frbp, ped/bikes	1.00	1.00	0.99	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Fr _t	1.00	1.00	0.85	1.00	0.97		1.00	1.00	0.85	1.00	0.98	
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1787	3574	1574	3467	3444		1743	1835	1591	1823	3551	
Flt Permitted	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (perm)	1787	3574	1574	3467	3444		1743	1835	1591	1823	3551	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	74	635	491	735	570	124	439	538	587	163	573	65
RTOR Reduction (vph)	0	0	51	0	14	0	0	0	26	0	7	0
Lane Group Flow (vph)	74	635	440	735	680	0	439	538	561	163	631	0
Confl. Bikes (#/hr)			2									
Heavy Vehicles (%)	1%	1%	2%	1%	2%	2%	2%	2%	0%	0%	1%	2%
Turn Type	Prot	NA	pm+ov	Prot	NA		Prot	NA	pm+ov	Prot	NA	
Protected Phases	5	2	3	1	6		3	8	1	7	4	
Permitted Phases			2						8			
Actuated Green, G (s)	7.4	24.4	55.1	26.0	43.0		30.7	42.2	68.2	11.9	23.4	
Effective Green, g (s)	7.4	24.4	55.1	26.0	43.0		30.7	42.2	68.2	11.9	23.4	
Actuated g/C Ratio	0.06	0.20	0.45	0.21	0.35		0.25	0.34	0.55	0.10	0.19	
Clearance Time (s)	4.0	5.5	4.0	4.0	5.5		4.0	5.5	4.0	4.0	5.5	
Vehicle Extension (s)	1.0	2.0	1.0	1.0	2.0		1.0	2.0	1.0	1.0	2.0	
Lane Grp Cap (vph)	107	706	702	729	1199		433	627	878	175	672	
v/s Ratio Prot	0.04	c0.18	0.16	c0.21	0.20		c0.25	c0.29	0.13	0.09	0.18	
v/s Ratio Perm			0.12						0.22			
v/c Ratio	0.69	0.90	0.63	1.01	0.57		1.01	0.86	0.64	0.93	0.94	
Uniform Delay, d1	56.9	48.4	26.3	48.8	32.7		46.4	37.9	19.1	55.4	49.3	
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	14.4	14.0	1.3	35.4	0.4		46.8	10.8	1.1	47.9	20.5	
Delay (s)	71.4	62.3	27.6	84.1	33.1		93.2	48.7	20.3	103.3	69.9	
Level of Service	E	E	C	F	C		F	D	C	F	E	
Approach Delay (s)			48.7		59.3			50.5			76.7	
Approach LOS			D		E			D			E	
Intersection Summary												
HCM 2000 Control Delay			56.8									E
HCM 2000 Volume to Capacity ratio			0.97									
Actuated Cycle Length (s)			123.5									19.0
Intersection Capacity Utilization			96.6%									F
Analysis Period (min)			15									
c Critical Lane Group												

HCM Unsigned Intersection Capacity Analysis

4: SW Strobel Rd & New Collector

01/19/2017

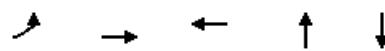


Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	1	1	1			1
Traffic Volume (veh/h)	28	0	100	50	0	56
Future Volume (Veh/h)	28	0	100	50	0	56
Sign Control	Stop		Free			Free
Grade	0%		0%			-4%
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	28	0	100	50	0	56
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None			None
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	181	125			150	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	181	125			150	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	97	100			100	
cM capacity (veh/h)	813	931			1444	
Direction, Lane #	WB 1	WB 2	NB 1	SB 1		
Volume Total	28	0	150	56		
Volume Left	28	0	0	0		
Volume Right	0	0	50	0		
cSH	813	1700	1700	1444		
Volume to Capacity	0.03	0.00	0.09	0.00		
Queue Length 95th (ft)	3	0	0	0		
Control Delay (s)	9.6	0.0	0.0	0.0		
Lane LOS	A	A				
Approach Delay (s)	9.6		0.0	0.0		
Approach LOS	A					
Intersection Summary						
Average Delay			1.1			
Intersection Capacity Utilization			18.3%		ICU Level of Service	
Analvsis Period (min)			15			A

Queues

1: Parking Lot/SW Tile Flat Rd & SW Scholls Ferry Rd

01/19/2017



Lane Group	EBL	EBT	WBT	NBT	SBT
Lane Group Flow (vph)	5	753	1011	1	446
v/c Ratio	0.06	0.62	0.88	0.00	0.96
Control Delay	54.2	13.4	26.9	0.0	67.0
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	54.2	13.4	26.9	0.0	67.0
Queue Length 50th (ft)	3	256	460	0	246
Queue Length 95th (ft)	18	452	#1069	0	#539
Internal Link Dist (ft)		230	1105	30	390
Turn Bay Length (ft)		130			
Base Capacity (vph)	82	1330	1145	412	466
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.06	0.57	0.88	0.00	0.96

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis
1: Parking Lot/SW Tile Flat Rd & SW Scholls Ferry Rd

01/19/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑			↔			↔			↔	
Traffic Volume (vph)	5	693	0	1	563	366	0	0	1	402	0	8
Future Volume (vph)	5	693	0	1	563	366	0	0	1	402	0	8
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	12	12	12	12	12	12	11	11	11	12	12	12
Grade (%)		4%			2%			0%			0%	
Total Lost time (s)	4.0	5.5			5.5			4.0			4.5	
Lane Util. Factor	1.00	1.00			1.00			1.00			1.00	
Fr _t	1.00	1.00			0.95			0.86			1.00	
Flt Protected	0.95	1.00			1.00			1.00			0.95	
Satd. Flow (prot)	1769	1808			1736			1589			1772	
Flt Permitted	0.95	1.00			1.00			1.00			0.95	
Satd. Flow (perm)	1769	1808			1735			1589			1772	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	5	753	0	1	612	398	0	0	1	437	0	9
RTOR Reduction (vph)	0	0	0	0	13	0	0	1	0	0	66	0
Lane Group Flow (vph)	5	753	0	0	998	0	0	0	0	0	380	0
Heavy Vehicles (%)	0%	3%	0%	0%	3%	2%	0%	0%	0%	2%	0%	0%
Turn Type	Prot	NA		Perm	NA			NA		Split	NA	
Protected Phases	5	2			6			8	8	4	4	
Permitted Phases					6							
Actuated Green, G (s)	0.9	75.7			70.8			0.7			24.6	
Effective Green, g (s)	0.9	75.7			70.8			0.7			24.6	
Actuated g/C Ratio	0.01	0.66			0.62			0.01			0.21	
Clearance Time (s)	4.0	5.5			5.5			4.0			4.5	
Vehicle Extension (s)	1.5	3.5			3.5			1.5			1.5	
Lane Grp Cap (vph)	13	1190			1068			9			379	
v/s Ratio Prot	0.00	c0.42						c0.00			c0.21	
v/s Ratio Perm					c0.57							
v/c Ratio	0.38	0.63			0.93			0.00			1.00	
Uniform Delay, d1	56.8	11.5			20.0			56.8			45.2	
Progression Factor	1.00	1.00			1.00			1.00			1.00	
Incremental Delay, d2	6.8	1.2			14.5			0.0			46.9	
Delay (s)	63.5	12.7			34.5			56.8			92.1	
Level of Service	E	B			C			E			F	
Approach Delay (s)		13.0			34.5			56.8			92.1	
Approach LOS		B			C			E			F	
Intersection Summary												
HCM 2000 Control Delay		38.7			HCM 2000 Level of Service			D				
HCM 2000 Volume to Capacity ratio		0.95										
Actuated Cycle Length (s)		115.0			Sum of lost time (s)			18.0				
Intersection Capacity Utilization		90.6%			ICU Level of Service			E				
Analysis Period (min)		15										
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis
 2: SW Vandermost Rd/SW Strobel Rd & SW Scholls Ferry Rd

01/19/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖ ↗	↑ ↘		↖ ↗	↑ ↗	↖ ↗		↖ ↗		↖ ↗	↑ ↗	
Traffic Volume (veh/h)	23	1059	3	3	912	129	2	0	14	72	1	14
Future Volume (Veh/h)	23	1059	3	3	912	129	2	0	14	72	1	14
Sign Control	Free			Free			Stop			Stop		
Grade	-4%			0%			-7%			-4%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	25	1151	3	3	991	140	2	0	15	78	1	15
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	None				TWLTL							
Median storage veh					2							
Upstream signal (ft)	1185											
pX, platoon unblocked				0.73			0.73	0.73	0.73	0.73	0.73	
vC, conflicting volume	1131			1154			2215	2340	1152	2213	2201	991
vC1, stage 1 conf vol							1202	1202		997	997	
vC2, stage 2 conf vol							1012	1137		1216	1204	
vCu, unblocked vol	1131			1024			2484	2655	1022	2481	2465	991
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)							6.1	5.5		6.1	5.5	
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	96			99			99	100	93	43	99	95
cM capacity (veh/h)	625			498			141	152	211	137	165	302
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	WB 3	NB 1	SB 1	SB 2				
Volume Total	25	1154	3	991	140	17	78	16				
Volume Left	25	0	3	0	0	2	78	0				
Volume Right	0	3	0	0	140	15	0	15				
cSH	625	1700	498	1700	1700	199	137	287				
Volume to Capacity	0.04	0.68	0.01	0.58	0.08	0.09	0.57	0.06				
Queue Length 95th (ft)	3	0	0	0	0	7	72	4				
Control Delay (s)	11.0	0.0	12.3	0.0	0.0	24.8	61.5	18.3				
Lane LOS	B		B			C	F	C				
Approach Delay (s)	0.2		0.0			24.8	54.1					
Approach LOS						C	F					
Intersection Summary												
Average Delay			2.4									
Intersection Capacity Utilization		73.2%			ICU Level of Service				D			
Analysis Period (min)			15									

Queues

3: SW Roy Rogers Rd/SW 175th Ave & SW Scholls Ferry Rd

01/19/2017



Lane Group	EBL	EBT	EBC	WBL	WBT	NBL	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	77	661	511	766	723	457	560	611	170	665
v/c Ratio	0.62	0.94	0.67	1.05	0.59	1.07	0.90	0.63	0.93	0.96
Control Delay	76.6	71.7	24.4	94.1	35.0	107.9	58.3	18.3	106.0	74.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	76.6	71.7	24.4	94.1	35.0	107.9	58.3	18.3	106.0	74.2
Queue Length 50th (ft)	61	276	242	~345	246	~407	427	276	137	276
Queue Length 95th (ft)	112	#389	361	#469	324	#614	#641	400	#274	#400
Internal Link Dist (ft)		686			809		447			409
Turn Bay Length (ft)	170		330	330		300		300	250	
Base Capacity (vph)	171	714	765	730	1227	427	621	968	186	697
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.45	0.93	0.67	1.05	0.59	1.07	0.90	0.63	0.91	0.95

Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis
 3: SW Roy Rogers Rd/SW 175th Ave & SW Scholls Ferry Rd

01/19/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑	↑	↑↑	↑↑		↑	↑	↑	↑	↑↑	
Traffic Volume (vph)	74	635	491	735	570	124	439	538	587	163	573	65
Future Volume (vph)	74	635	491	735	570	124	439	538	587	163	573	65
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)		0%			0%			3%			-2%	
Total Lost time (s)	4.0	5.5	4.0	4.0	5.5		4.0	5.5	4.0	4.0	5.5	
Lane Util. Factor	1.00	0.95	1.00	0.97	0.95		1.00	1.00	1.00	1.00	0.95	
Frbp, ped/bikes	1.00	1.00	0.99	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Fr _t	1.00	1.00	0.85	1.00	0.97		1.00	1.00	0.85	1.00	0.98	
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1787	3574	1574	3467	3444		1743	1835	1591	1823	3551	
Flt Permitted	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (perm)	1787	3574	1574	3467	3444		1743	1835	1591	1823	3551	
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	77	661	511	766	594	129	457	560	611	170	597	68
RTOR Reduction (vph)	0	0	51	0	14	0	0	0	26	0	7	0
Lane Group Flow (vph)	77	661	460	766	709	0	457	560	585	170	658	0
Confl. Bikes (#/hr)			2									
Heavy Vehicles (%)	1%	1%	2%	1%	2%	2%	2%	2%	0%	0%	1%	2%
Turn Type	Prot	NA	pm+ov	Prot	NA		Prot	NA	pm+ov	Prot	NA	
Protected Phases	5	2	3	1	6		3	8	1	7	4	
Permitted Phases			2						8			
Actuated Green, G (s)	7.6	25.1	55.4	26.0	43.5		30.3	41.8	67.8	12.4	23.9	
Effective Green, g (s)	7.6	25.1	55.4	26.0	43.5		30.3	41.8	67.8	12.4	23.9	
Actuated g/C Ratio	0.06	0.20	0.45	0.21	0.35		0.24	0.34	0.55	0.10	0.19	
Clearance Time (s)	4.0	5.5	4.0	4.0	5.5		4.0	5.5	4.0	4.0	5.5	
Vehicle Extension (s)	1.0	2.0	1.0	1.0	2.0		1.0	2.0	1.0	1.0	2.0	
Lane Grp Cap (vph)	109	721	701	725	1205		424	617	867	181	682	
v/s Ratio Prot	0.04	c0.18	0.16	c0.22	0.21		c0.26	c0.31	0.14	0.09	0.19	
v/s Ratio Perm			0.13						0.23			
v/c Ratio	0.71	0.92	0.66	1.06	0.59		1.08	0.91	0.67	0.94	0.96	
Uniform Delay, d1	57.3	48.6	27.0	49.1	33.1		47.0	39.4	20.3	55.6	49.8	
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	15.6	16.1	1.7	49.4	0.5		66.2	16.8	1.6	48.5	25.6	
Delay (s)	72.9	64.7	28.7	98.5	33.5		113.2	56.2	22.0	104.0	75.3	
Level of Service	E	E	C	F	C		F	E	C	F	E	
Approach Delay (s)		50.5			67.0			59.3			81.2	
Approach LOS		D			E			E			F	
Intersection Summary												
HCM 2000 Control Delay		62.9										E
HCM 2000 Volume to Capacity ratio		1.02										
Actuated Cycle Length (s)		124.3										19.0
Intersection Capacity Utilization		96.6%										F
Analysis Period (min)		15										
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis

4: SW Strobel Rd & New Collector

01/19/2017

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↖ ↗ ↘ ↖ ↙ ↘					
Traffic Volume (veh/h)	28	0	100	50	0	56
Future Volume (Veh/h)	28	0	100	50	0	56
Sign Control	Stop		Free			Free
Grade	0%		0%			-4%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	30	0	109	54	0	61
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None			None
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	197	136		163		
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	197	136		163		
tC, single (s)	6.4	6.2		4.1		
tC, 2 stage (s)						
tF (s)	3.5	3.3		2.2		
p0 queue free %	96	100		100		
cM capacity (veh/h)	796	918		1428		
Direction, Lane #	WB 1	WB 2	NB 1	SB 1		
Volume Total	30	0	163	61		
Volume Left	30	0	0	0		
Volume Right	0	0	54	0		
cSH	796	1700	1700	1428		
Volume to Capacity	0.04	0.00	0.10	0.00		
Queue Length 95th (ft)	3	0	0	0		
Control Delay (s)	9.7	0.0	0.0	0.0		
Lane LOS	A	A				
Approach Delay (s)	9.7		0.0	0.0		
Approach LOS	A					
Intersection Summary						
Average Delay			1.1			
Intersection Capacity Utilization		18.3%		ICU Level of Service		A
Analysis Period (min)			15			